

Submitted to DTE Energy, Inc.

Submitted by AECOM 27777 Franklin Road, Suite 2000 Southfield, MI 48034 August 2019 AECOM Project No. 60516675

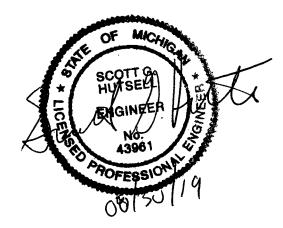
## **History of Construction Report**

# Inactive Bottom Ash Impoundment DTE Monroe Power Plant

March 2018 Revision 1: August 2019

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#### 1 Introduction

The United States Environmental Protection Agency (EPA) promulgated the Resource Conservation and Recovery Act (RCRA) Coal Combustion Residuals (CCR) Rule ("CCR RCRA Rule") on April 17, 2015. The CCR RCRA Rule requires that owners or operators of existing CCR surface impoundments with a height of five feet or more and a storage volume of 20 acre-feet or more compile a history of construction, which shall contain, to the extent feasible, the information specified in 40 CFR 257.73 (c)(1)(i) through (xii). The history of construction, and any revisions of it, as required by 40 CFR 257.73(c) shall be placed in the operating record and shall be maintained until the CCR unit completes closure of the unit in accordance with 40 CFR 257.102 [40 CFR 257.105(f)(9)].

#### 2 History of Construction Report

This following subsections provide the information required by the Coal Combustion Residual (CCR) Rule (40 Code of Federal Regulations (CFR) Part 257) (the "Rule"). Section §257.73(c)(l) of the CCR Rule states that "...the owner or operator of the CCR unit must compile a history of construction, which shall contain, to the extent feasible, the information specified in paragraphs (c)(1)(i) through (xii) of this section."

#### 2.1 Name and Address

The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

DTE Electric Company

**Michael Twomley** 

3500 East Front St.

Monroe, MI 48161

Name of CCR Surface Impoundment: Monroe Inactive Bottom Ash Impoundment

State Assigned Identification Number: None

#### 2.2 Location

The USGS quadrangle map showing Monroe, Michigan (Stony Point quadrangle) with the Inactive Bottom Ash Impoundment is presented in **Figure 1**.

#### 2.3 Statement of Purpose

The Inactive Bottom Ash Impoundment consists of an embankment constructed primarily with rock fill and earth spoils generated during construction of the Monroe Power Plant. The impoundment was constructed on the existing natural ground surface, which also forms the liner and is primarily used for shore protection. The height of the embankment (from the lowest toe elevation to the top of embankment) is approximately 4 feet. A road along the top of the crest has a minimum width of approximately 12 feet and is 20 or more feet wide along the eastern side abutting Lake Erie, which was constructed with additional rock armament for shoreline protection. The northern boundary of the Inactive Bottom Ash Impoundment is the geosynthetic lined process wastewater ditch (anticipated construction completion in late 2019) which will re-route all plant process waste water around the Inactive Bottom Ash Impoundment to the process waste and stormwater basin to the South.

The southern boundary of the Inactive Bottom Ash Impoundment is a crushed rock embankment that separates Inactive Bottom Ash Impoundment from the process waste and stormwater basin to the South.

The aerial chronology figure in **Appendix A** shows the progression of bottom ash placement at the Inactive Bottom Ash Impoundment over time.

#### 2.4 Name and size of Watershed

According to the EPA WATERS Geoviewer website (USEPA 2018), the Inactive Bottom Ash Impoundment and the Monroe Power Plant are located within the Plum Creek Subwatershed, which encompasses approximately 23,000 acres.

#### 2.5 Engineering properties of foundation materials

A subsurface exploration was performed at the Inactive Bottom Ash Impoundment by AECOM in November 2017 in support of the slope stability analysis in the Safety Factor Assessment.

A total of nine stratigraphic units were identified as a result of the subsurface explorations. Seven of the units were relevant to this slope stability analysis. Brief descriptions of each relevant unit are as follows:

• **Fill – Perimeter Dikes**: The perimeter dikes were found to be constructed of fill material, consisting of a variety of interlayered materials including sand (USCS type SP, SP-SM, SW), gravel (GP), silty sand (SM), lean clay (CL), silty clay (CL-ML), organic silt as topsoil (OL), asphalt and cobbles.

• **Fill – Divider Berm**: The divider berm was found to be constructed of fill material consisting of moist to wet, light to dark gray crushed limestone classified as sandy gravel (GP and GW-GM) with trace amounts of silt.

• Silt and Clay: A unit of soft cohesive silt and clay was encountered below the fill materials at both the divider berm and outer dikes and consisted of cohesive elastic silt (MH) with organics and cohesive silt (ML) with organics, lean clay (CL) and silty clay (CL-ML) with variable proportions of organics, and minor proportions of non-cohesive silt (ML) with organics. The unit contained moderate organic content, which generally decreased and eventually disappeared with depth.

• Loose Sand: A unit of loose sand was encountered below the soft cohesive silt and clay unit and consisted of generally wet, gray poorly graded sand (SP), poorly graded sand with silt (SP-SM), silty sand (SM), clayey sand (SC) and silty clayey sand (SC-SM).

• **Sand and Gravel**: A unit of sand and gravel was encountered below the loose sand and consisted of generally wet, gray and brown interbedded silty sand (SM), clayey sand (SC), well-graded sand with silt and gravel (SW-SM) and gravel layers classified as poorly graded gravel (GP) and well graded sand with silt and gravel GW-GM).

 $\cdot$  Till: Clay till was encountered at all the borings beneath the sand and gravel unit and consisted of very stiff to hard lean Clay (CL) with interbedded minor proportions of silty Clay (CL-ML) and Silt (ML). The clay material contained variable proportions of gravel, sand, silt and clay.

 $\cdot$  **Bedrock:** Bedrock primarily consisted of light gray moderately weathered limestone and/or dolomite. In general, the bedrock was found to generally slope downward from west to east (towards Lake Erie), and is shallowest at a possible bedrock ridge noted near the existing overflow weir.

Sections showing subsurface foundation materials are presented in Appendix B.

#### 2.6 Engineering properties of materials used in construction

Records of material testing performed during construction of the basin and perimeter dikes from 1968 to 1970 are not available. Presented below are engineering properties of the differing soil materials collected during the geotechnical investigation in November of 2017.

Engineering properties of materials used in construction							
Unit Weight (pcf)			Peak Total (undrained) Shear Strength Parameters				
	c' (psf)	Φ' (°)	c (psf)	Φ (°)			
120	0	30	0	30			
130	0	33	0	33			
100	110	32	220	16.4			
120	0	28	0	28			
120	0	33	0	33			
130	4,000	0	4,000	0			
Bedrock Assumed to be impenetrable in the slope stability models							
	Unit Weight (pcf) 120 130 100 120 120 120 130	Unit Weight (pcf)         Peak Effect Shear Streng           120         0           130         0           100         110           120         0           130         0           130         0           120         0           130         110           120         0           120         0           130         4,000	Unit Weight (pcf)         Peak Effective (drained) Shear Strength Parameters           120 $c' (psf)$ $\Phi' (°)$ 120         0         30           130         0         33           100         110         32           120         0         33           130         0         33           130         0         33           130         0         33           130         0         33           130         0         0	Unit Weight (pcf)         Peak Effective (drained) Shear Strength Parameters         Peak Total Shear Strength           120 $0'$ (°)         c (psf)           120         0         30         0           130         0         33         0           120         0         33         0           130         0         32         220           120         0         28         0           120         0         33         0           130         4,000         0         4,000			

1. Engineering parameters estimated based on correlations with SPT N-values, pocket penetrometer readings, our experience with these soil types and engineering judgement.

 Shear strength parameters estimated based on interpretation of Consolidated Undrained (CU) Triaxial test data in accordance with the Modified Mohr-Coulomb plot (a p-q and p'-q plot) procedures, as described in Appendix D of the United States Corps of Engineers Manual EM-1110-2-1902 "Slope Stability."

#### 2.7 Detailed Dimensional Drawings of the CCR Unit

Drawings provided in **Appendix B** show the extent of the Inactive Bottom Ash Impoundment and the location of cross-sections showing the impoundment and subsurface conditions.

#### 2.8 Description of Existing Instrumentation

The locations of the monitoring wells installed for the groundwater program are included as **Figure 2** of this report.

#### 2.9 Area Capacity Curves

Area capacity curves for the Inactive Bottom Ash Impoundment were calculated by AECOM and are included in the "Inflow Design Flood Control System Plan".

#### 2.10 Spillway and Diversion Design Features

The Inactive Bottom Ash Impoundment discharge structure is an over/under weir constructed at a fixed elevation of 575 feet (datum not indicated on DTE Drawing No. 6C695-270) at the west part of the pond. The flow discharges into the Discharge Canal at NPDES Monitoring Point 001B.

#### 2.11 Construction Specifications and provisions for repair

DTE conducts periodic surveillance and maintenance of the Inactive Bottom Ash Impoundment in accordance with DTE's internal practice.

#### 2.12 Knowledge of Structural Instability

DTE personnel were questioned to identify historical information that could have been indicative of structural instability. Those DTE personnel revealed that there are no records or knowledge of structural instability associated with the Inactive Bottom Ash Impoundment.

#### 3 References

AECOM (2018) "Inflow Design Flood Control System"

AECOM (2018). "Safety Factor Assessment"

GZA Geoenvironmental (2011) "Final Report, Round 7 Dam Assessment, DTE Energy Monroe Power Plan, Fly Ash Basin and Bottom Ash Stormwater Pond"

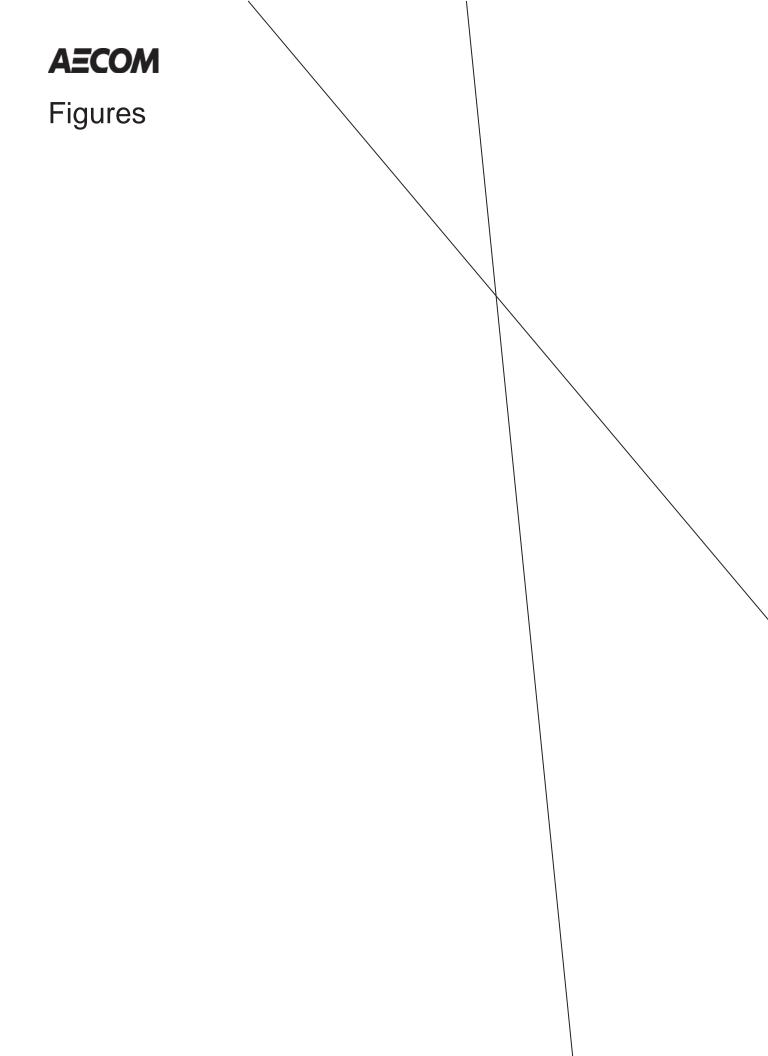
U.S. Environmental Protection Agency [USEPA]. (2015). Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments. 40 CFR §257. Federal Register 80, Subpart D, April 17, 2015.

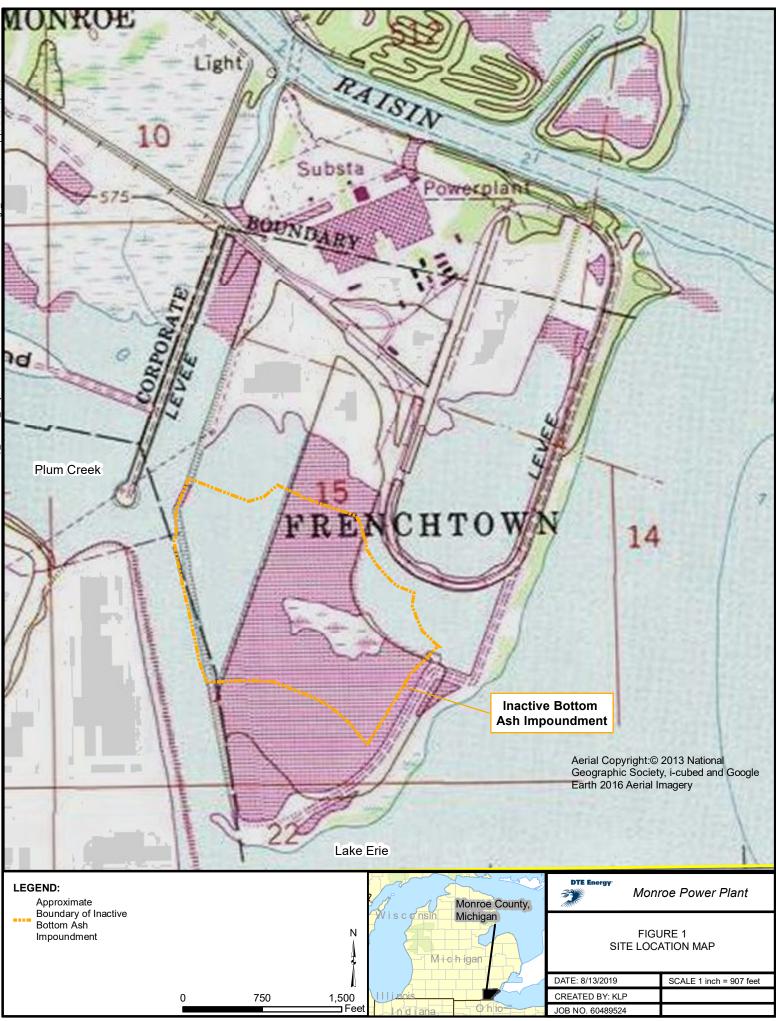
U.S. Environmental Protection Agency [USEPA]. (2018) Waters Geoviewer Mapper (2016). https://www.epa.gov/waterdata/waters-geoviewer

**Revision Log** The table below provides a description of revisions to the History of Construction Report.

REVISION #	REVISION DATE	DESCRIPTION OF REVISION
1	08/30/2019	Changed text on the cover page and pages 2-1, 2-2, 2-3, and 2-4, and updated Figures 1 and 2.



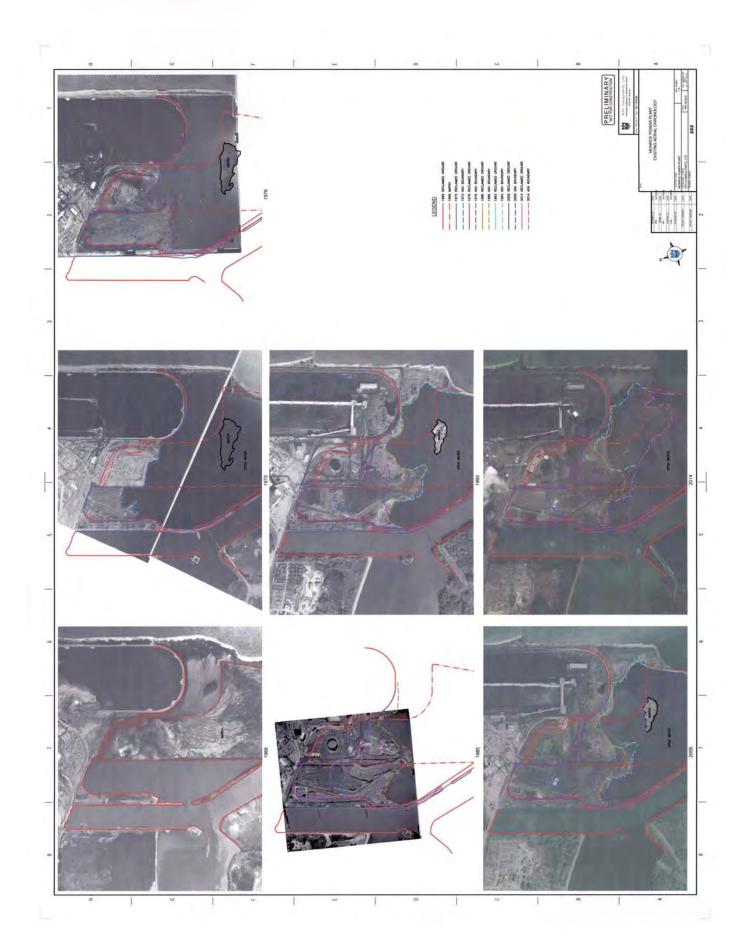






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### Appendix A Historical Aerial Photography



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### Appendix B Construction and Site Drawings

