

Run-on/Run-off Control System Plan:
Coal Combustion Residuals (CCR)
Disposal Facility – Monroe Vertical
Extension Landfill

Prepared for:



October 15, 2021

Run-On/Run-Off Control System Plan:
Coal Combustion Residuals (CCR) Disposal Facility – Monroe
Vertical Extension Landfill

Prepared for:

DTE Energy
One Energy Plaza
Detroit, MI 48226

Prepared by:

AECOM

Project No. 60662907

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List of Acronyms and Abbreviations

3H:1V	3 horizontal feet to 1 vertical foot	H&H	hydrologic and hydraulic
CCR	coal combustion residuals	NPDES	National Pollutant Discharge Elimination System
DTE	DTE Energy	MDEQ	Michigan Department of Environmental Quality
EPA	U.S. Environmental Protection Agency	MPP	Monroe Power Plant
EGLE	Michigan Department of Environment, Great Lakes, and Energy	Plan	Run-on/Run-off Control System Plan
FGD	flue gas desulfurization	WSE	water surface elevation

1 Introduction

This Run-on/Run-off Control System Plan (Plan) was prepared for an existing coal combustion residuals (CCR) disposal facility located at the DTE Electric Company (DTE) Monroe Power Plant (MPP) in Monroe, Michigan. This Plan serves as the five-year update to the initial Plan issued on October 17, 2016. The CCR disposal facility consists of a landfill located on top of an existing ash basin. The disposal facility currently accepts bottom ash, fly ash, flue gas desulfurization (FGD) sludge, scrubber water sludge solidified with fly ash or bottom ash, synthetic gypsum, and inert materials.

The plan was prepared in accordance with 40 CFR Part 257 and specifically addresses the requirements under Subpart D, §257.81 of the U.S. Environmental Protection Agency (EPA) CCR Rule. It is noted that the disposal facility is an existing landfill operating under an operating license approved by the Michigan Department of Environment, Great Lakes, and Energy (EGLE), on December 4, 2019. The operating license was granted in accordance with Part 115 of the Natural Resources and Protection Act of 1994, as amended, which adopts §257.81 by reference. Accordingly, run-on and run-off control system requirements for the disposal facility must meet or exceed those of the CCR Rule.

1.1 Site Location and Description

The CCR disposal facility is located on land currently owned by DTE at the MPP. The landfill is operated by DTE. The MPP is located at 3500 East Front Street in Monroe Township, on the west bank of Lake Erie, approximately one and a half miles east of Interstate Highway 75 (I-75). While the landfill operations have continued since the initial Plan was issued, the run-on/run-off control system at the site has generally remained unchanged. The CCR disposal facility site consists of the landfill within the limits of an existing 410-acre Ash Basin. The disposal site is located approximately 1 mile southwest of MPP and is bounded by Lake Erie and the plant discharge channel on the east, I-75 on the west, a residential property and Plum Creek on the north, and an agricultural field on the south.

The plant has been in operation since 1971, and currently consists of four coal-fired electric generating units producing for a total output of approximately 3,400 megawatts (MW). Management of CCR at this facility currently consists of a combination of wet sluicing material to the ash basin and placing dry material into the landfill.

1.2 Description of CCR Landfill Operations

The landfill is designed to cover approximately 79 acres and be constructed in two phases sequentially moving west to east. The depth of fly ash was identified to be approximately 40 feet below the preconstruction ground surface. Graded sluiced fly ash makes up the subgrade for the landfill. A pore pressure relief layer is constructed on top of the subgrade. This layer consists of the following from bottom to top: 24-inches of bottom ash or equivalent, a network of perforated piping encased in filter fabric, a layer of non-woven geotextile, and a 6-inch embedment layer of fill. Pore water that exists below the subgrade discharges through the network of perforated pipes into perimeter swales. Likewise, stormwater is routed through the same pipe network as the

landfill is being filled. As final grades are met, surface water runoff will be directed to control berms and to perimeter swales. The perimeter swales convey all water to the ash basin, where the water is ultimately discharged under a Michigan National Pollutant Discharge Elimination System (NPDES) permit number MI0001848, previously issued by EGLE on April 4, 2014.

1.3 CCR Rule Requirements

(40 CFR) 257.81(a) *The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must maintain:*

(1) *A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm; and*

(2) *A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm.*

(b) Run-off from the active portion of the CCR unit must be handled in accordance with the surface water requirements under § 257.3–3.

The MPP disposal facility is an existing landfill that was designed to incorporate run-on and run-off controls systems, which prevent flow onto and control flow from the active portion of the unit during a 24-hour, 25-year storm.

1.4 Plan Content

(40 CFR) 257.81(c) *Run-on and run-off control system plan—*

(1) *Content of the plan. The owner or operator must prepare initial and periodic run-on and run-off control system plans for the CCR unit according to the timeframes specified in paragraphs (c)(3) and (4) of this section. These plans must document how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of this section. Each plan must be supported by appropriate engineering calculations. The owner or operator has completed the initial run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by § 257.105(g)(3).*

This Plan is the five-year update to the initial run-on/run-off control plan, and it describes how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of the CCR Rule. A certification statement from a qualified professional engineer verifying that this Plan meets the requirements of this section § 257.81, In accordance with §257.81(c)(1), this Plan will be amended each time there is a change in conditions that substantially affect the written plan in effect.

1.5 Documents Reviewed

Background information, design basis information, and other data used in preparing this plan have been provided to AECOM by DTE. AECOM is not responsible for the accuracy of the documents

prepared by others and has prepared this plan by practicing good engineering judgement based upon the best available information. The following documents and design drawings were reviewed in the preparation of this plan:

- AECOM, October 17, 2016. *Run-on/Run-off Control System Plan For Coal Combustion Residuals (CCR) Disposal Facility – Monroe Fly Ash Basin Vertical Extension.*
- Commonwealth Associates Inc., June 1977. *Layout – On-site Fly Ash Disposal Facility Discharge Structure, Design Drawings.*
- Detroit Edison's Surveying Services, June 2016. *Discharge Structure – Existing & Proposed Conditions, Design Drawings.*
- Geosyntec Consultants, January 2021. *2020 Annual Inspection Report: Vertical Extension Landfill.*
- Golder Associates Inc., April 2015. *Permit Modification, DTE Energy Monroe Power Plant Ash Basin*
- Golder Associates Inc., September 2015. *DTE Energy Monroe Fly Ash Basin, 2015 Phase I Record Drawings*
- Golder Associates Inc., November 2017. *DTE Energy Monroe Fly Ash Basin, 2017 Phase 2 Record Drawings*

Additional information on the references utilized for this plan can be found in **Section 4**.

2 Overview of Run-on/Run-off Control Systems

The run-on and run-off control systems share multiple common control measures and are both required to control the peak flows resulting from a 25-year/24-hour storm. Due to these similarities, one hydrologic and hydraulic (H&H) model was constructed in HydroCAD (version 10.00-20) to analyze both systems in order to evaluate the run-on and run-off control systems' abilities to control the design storm. The H&H model utilized in the initial run-on/run-off control plan was found to sufficiently represent current site conditions, and therefore was used in this Plan with relatively minor changes¹. The resulting output from this model can be found in **Appendix B2**. The components that make up the run-on and run-off control systems are described in detail below.

2.1 Run-on Controls

Run-on controls consist of diversion berms which divert stormwater away from active disposal areas and also direct surface water to receiving flumes or drainage ditches. In addition, the proposed cap system is graded at a minimum of 5% to drain stormwater flows away from active portions of the landfill. The active area of any phase will be minimized to reduce contact water and the potential for fugitive dust emissions. Furthermore, the areas immediately outside of the landfill's perimeter are sloped away from the perimeter ditch system, preventing run-on from adjacent land from entering the facility.

2.2 Permanent Run-off Management Features

Permanent run-off management features are shown on Sheet 6 of the *2015 Phase 1 Record Drawings* and Sheet 6 of the *2017 Phase 2 Record Drawings* (**Appendix A2**). Associated details are provided on Sheets 9 and 10 of the *2015 Phase 1 Record Drawings* and Sheets 9 and 10 of the *Phase 2 Record Drawings* (**Appendix A2**). The cap system's grade ranges from a slope of 5% at the top to 4 horizontal feet to 1 vertical foot (4H:1V) along the perimeter. Surface water control berms sloped at 2% are designed on the 4H:1V portion of the cap. These berms create small v-shaped channels 2 feet deep with 3H:1V side slopes that direct stormwater flows into the perimeter swales while minimizing erosive velocities. The perimeter swales are sloped at approximately 0.1% with a 12-foot bottom width and 3H:1V side slopes and they direct stormwater flows into the existing ash basin.

All permanent run-off measures are designed to collect and control the peak flow resulting from a 25-year/24-hour storm under final design conditions. Supporting calculations for the surface water control structures are provided in **Appendix B2**.

2.3 Erosion Control

The entire cap system of the landfill is designed with a minimum of 6 inches of vegetative cover. This vegetative cover is sufficient to minimize potential erosion on all areas of the cap system.

¹ Note that the H&H model represents a combination of existing conditions around the perimeter of the landfill and proposed final buildout/closed conditions internal to the landfill, which is the most conservative scenario.

2.4 Collection and Holding Facilities

Finally, all resultant stormwater flows are conveyed from the perimeter swales into the existing ash basin. Stormwater runoff settles within the ash basin and ultimately discharges through a permitted NPDES outfall at its eastern end (permit number MI0001848 issued by EGLE. The existing water surface elevation (WSE) of the ash basin is approximately 609.0 feet. The starting WSE of 609.0 feet was used to model the tailwater conditions for the perimeter swales.

3 Frequency for Revising the Plan

(40 CFR) 257.81(c)(4). *The owner or operator of the CCR unit must prepare periodic run-on and runoff control system plans required by paragraph (c)(1) of this section every five years. The date of completing the initial plan is the basis for establishing the deadline to complete the first subsequent plan. The owner or operator may complete any required plan prior to the required deadline provided the owner or operator places the completed plan into the facility's operating record within a reasonable amount of time. In all cases, the deadline for completing a subsequent plan is based on the date of completing the previous plan. For purposes of this paragraph (c)(4), the owner or operator has completed a periodic run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by § 257.105(g)(3).*

This Plan represents the first five-year update subsequent to the initial run-on/run-off control system plan published in 2016 as outlined in §257.81(c)(4). As such, the initial run-on/run-off control system plan is superseded by this Plan, and DTE will place it in the facility's operating record.

DTE will continue to update periodic run-on and runoff control system plans every five years and will place the Plan in the facility's operating record. DTE will obtain a certification from a qualified professional engineer stating that the periodic run-on and run-off control system plans meet the requirements of this section.

4 References

AECOM, October 17, 2016. *Run-on/Run-off Control System Plan For Coal Combustion Residuals (CCR) Disposal Facility – Monroe Fly Ash Basin Vertical Extension.*

Commonwealth Associates Inc., June 1977. *Layout – On-site Fly Ash Disposal Facility Discharge Structure, Design Drawings.*

Detroit Edison's Surveying Services, June 2016. *Discharge Structure – Existing & Proposed Conditions, Design Drawings.*

Geosyntec Consultants, January 2021. *2020 Annual Inspection Report: Vertical Extension Landfill.*

Golder Associates Inc., April 2015. *Permit Modification, DTE Energy Monroe Power Plant Ash Basin*

Golder Associates Inc., March 2015. *DTE Energy Monroe Fly Ash Basin, Construction Permit Application Modification, Design Drawings*

Golder Associates Inc., May 2015. *DTE Energy Monroe Fly Ash Basin, 2015 Construction Drawing, Design Drawings*

HydroCAD Software Solutions LLC 2017. *HydroCAD, Version 10.00-20 Computer Program.*

National Oceanic and Atmospheric Administration (NOAA) 2017. *Point Precipitation Frequency Estimates, Atlas 14, Volume 8, Version 2, for Monroe, Michigan.*

APPENDIX A: PLAN DRAWINGS & CERTIFICATION

Appendix A.1: CCR Rule Engineer's Certification

Certification Statement 40 CFR § 257.81(c)(5) –Run-on and Run-Off Control System Plan for an Existing CCR Landfill

CCR Unit: DTE Energy Range Road Landfill

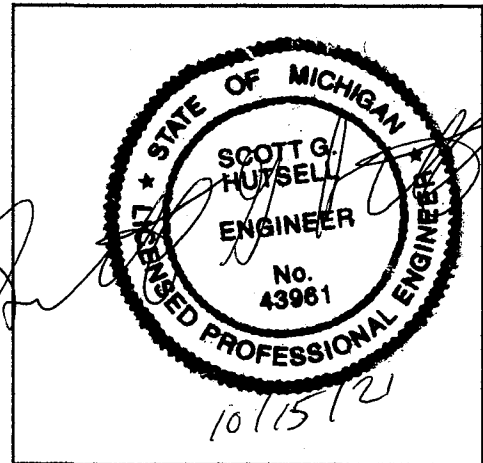
I, Scott G. Hutsell, being a Registered Professional Engineer in good standing in the State of Michigan, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the information contained in the run-on and run-off control system plan dated October 15, 2021 meets the requirements of 40 CFR § 257.81.

SCOTT G. HUTSELL

Printed Name

10/15/21

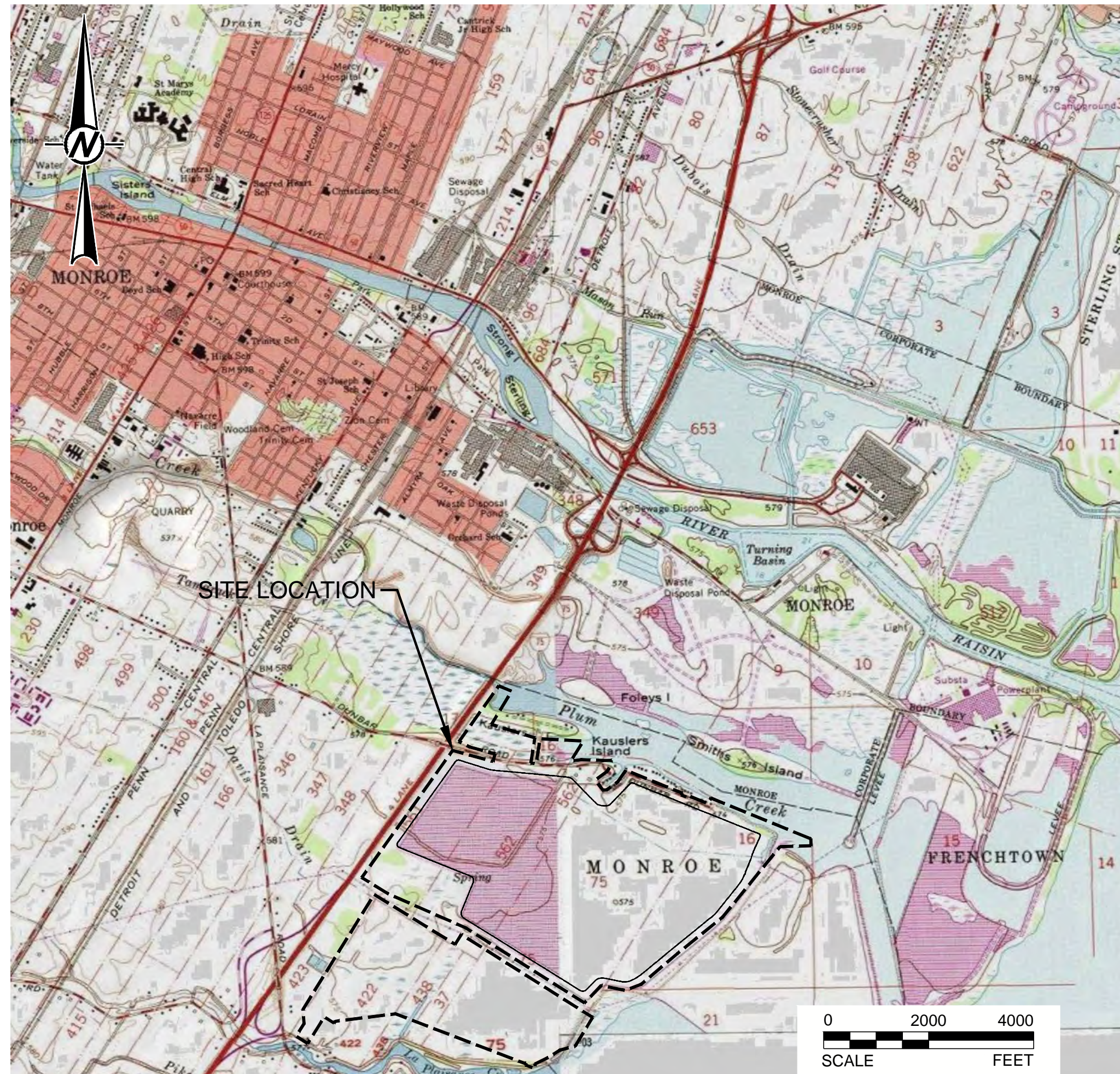
Date



APPENDIX B: PLAN DRAWINGS & CERTIFICATION

Appendix A.2: Historic Design Drawings

DTE ENERGY MONROE FLY ASH BASIN 2015 PHASE 1 RECORD DRAWINGS



Prepared for:
DTE ENERGY
One Energy Plaza
Detroit, MI 48226

Prepared by:
Golder Associates Inc.
15851 S. US 27 Suite 50
Lansing, Michigan USA 48906

INDEX OF DRAWINGS			
Sheet Number	Drawing Title	Current Revision	Date
1	Title Sheet	0	
2	Legend, References and General Notes	0	
3	Site Plan	0	
4	Record Top of Subgrade Plan	0	
5	Record Pore Water Relief Piping Plan	0	
6	Record Pore Water Relief System and Berm Plan	0	
7	Construction Control / QA/QC Points	0	
8	Record Monitoring Locations for Phase 1	0	
9	General Details - Sheet 1	0	
10	General Details - Sheet 2	0	

RECORD DRAWING

CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

CONSULTANT



YYYY-MM-DD	2015-09-10
PREPARED	JJS
DESIGN	JJS
REVIEW	JJS
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
TITLE SHEET




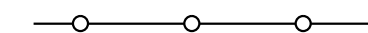












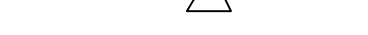










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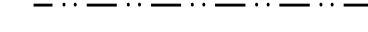


Rev.

FIGURE
1

LEGEND

-  PROPERTY BOUNDARY
-  ASH BASIN EXISTING TOPOGRAPHY (1' AND 5' CONTOURS)
-  WATER BOUNDARY
-  FENCE LINE
-  ELECTRIC TOWER
-  ELECTRIC POLE
-  UNDERGROUND ELECTRIC LINE
-  CONSUMERS ENERGY ELECTRIC LINE
-  ITC ELECTRIC LINE
-  RIGHT OF WAY
-  DRAINAGE DIRECTION
-  SAMPLE LOCATION
-  DECOMMISSIONED SLURRY PIPELINES
-  ACTIVE SLURRY PIPELINES
-  WETLAND
-  STRUCTURE (RESIDENCE, BUSINESS)
-  CULVERT
-  BENCHMARK LOCATION
-  WATER LINE
-  VEHICLE CROSSING
-  PROPOSED DRAINAGE CHANNEL
-  PROPOSED PERFORATED, FILTER SOCKED, 0.000' TO 0.000' DIA. AND 0.000' TO 0.000' DIA. D
-  AREA OF CLAY COVER
-  AREA OF FILL TICK MARK
-  AREA OF CUT TICK MARK
-  CONTROL POINT
-  CULVERT

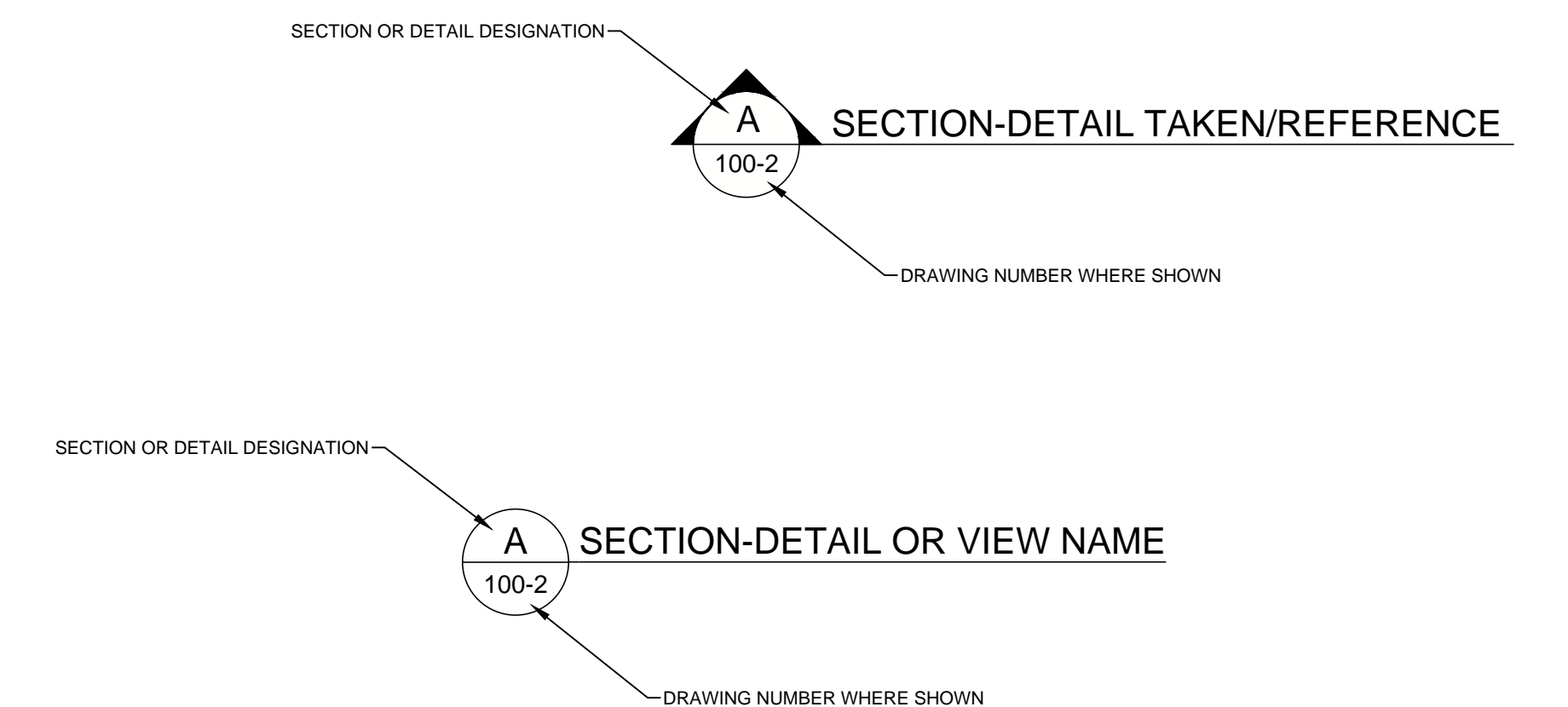
MATERIALS LEGEND

-  SLUICED FLY ASH
-  BOTTOM ASH OR EQUIVALENT
-  8 oz/sy GEOTEXTILE
-  LOW PERMEABILITY SOILS COVER MATERIAL
-  STRUCTURAL FILL
-  TOPSOIL

GENERAL NOTES

1. EXISTING TOPOGRAPHY IS AN AMALGAM OF GRADES FROM 2009 BASE MAP (DRAWING 6SE-0695-070-REV B) AND AS-BUILT GRADES FROM MITIGATED SECTIONS OF THE EMBANKMENT, PROVIDED BY DTE, DRAWING CREATED BY GEOSYNTEC CONSULTANTS, DATED 02/08/2013.
2. HORIZONTAL GRID COORDINATE SYSTEM UNITS ARE IN FEET AND REFERENCED TO MICHIGAN SOUTH STATE PLANE COORDINATES SYSTEM (NAD83). ELEVATIONS ARE IN FEET AND REFERENCED TO NATIONAL GEODETIC VERTICAL DATUM (NGVD29). NGVD29 DATUM IS 1.47 FT HIGHER THAN THE PLANT DATUM.
3. AREA OF CLAY COVER ADAPTED FROM "ATTACHMENT A" OF THE DTE ELECTRIC COMPANY SOLID WASTE OPERATING LICENSE APPLICATION, DATED DECEMBER 3, 2014.

SECTION, DETAIL AND VIEW DESIGNATIONS



RECORD DRAWING

CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

CONSULTANT

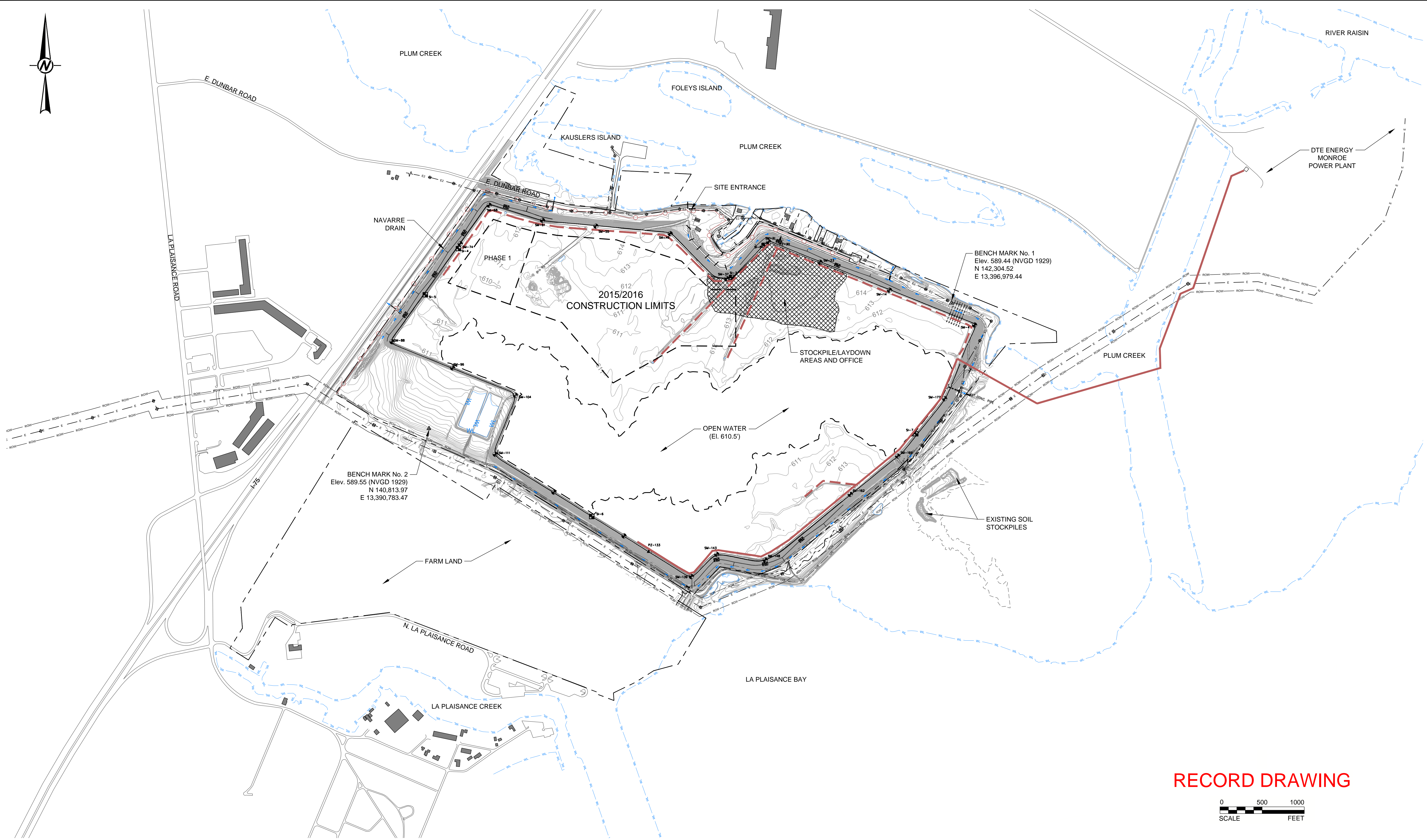
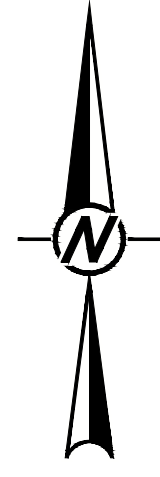


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PREPARED	JJS
DESIGN	JJS
REVIEW	JJS
APPROVED	DML

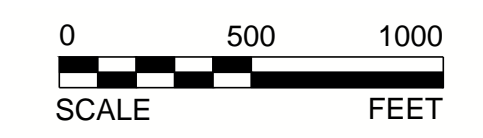
PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
LEGEND, REFERENCES AND GENERAL NOTES

PROJECT No.	CONTROL	Rev.	FIGURE
1521809B	A		2



RECORD DRAWING



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MONROE POWER PLANT
MONROE, MI
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DESIGN	JJS
REVIEW	JJS
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
SITE PLAN

PROJECT No.
1521809B

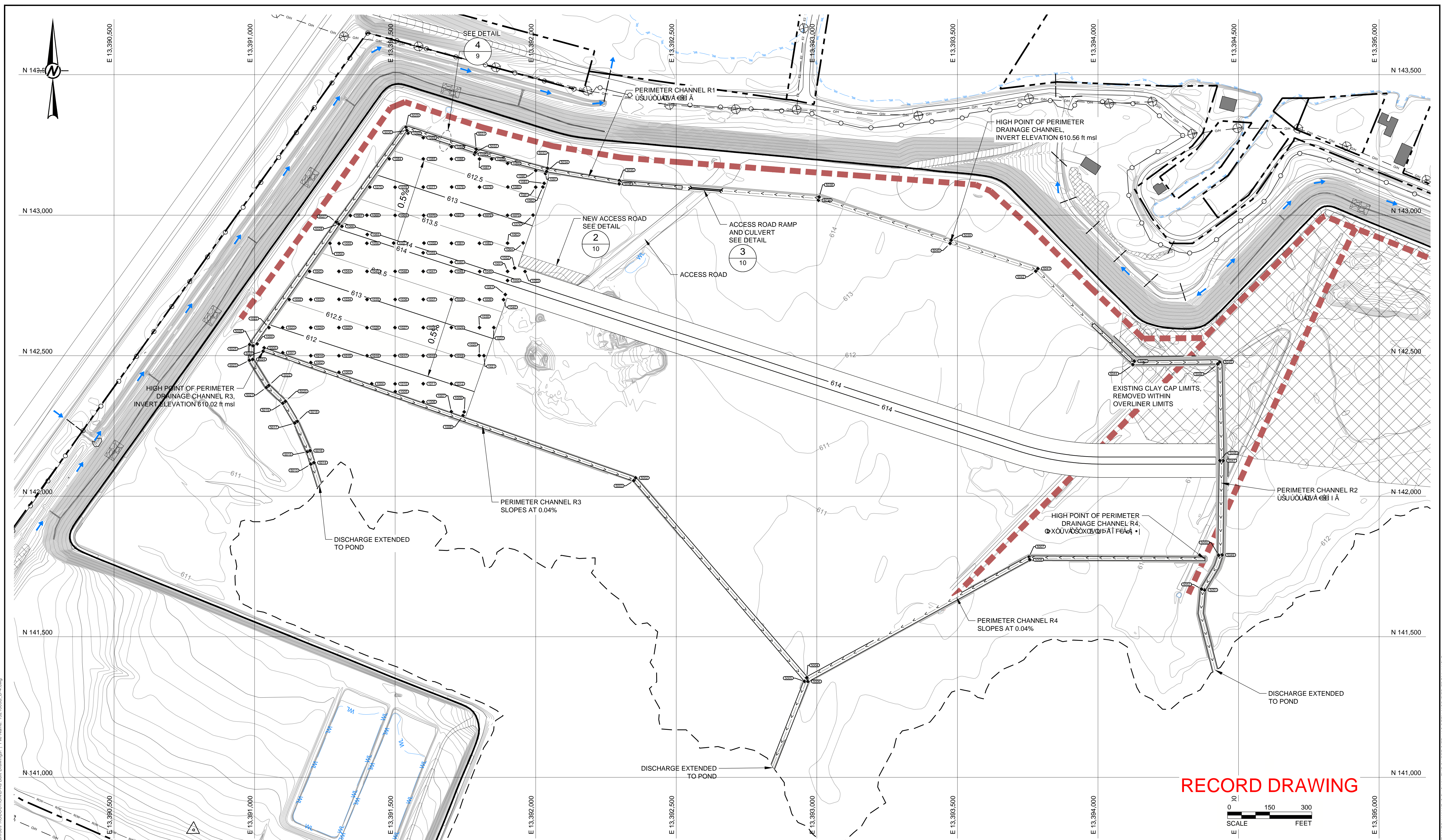
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FIGURE
3

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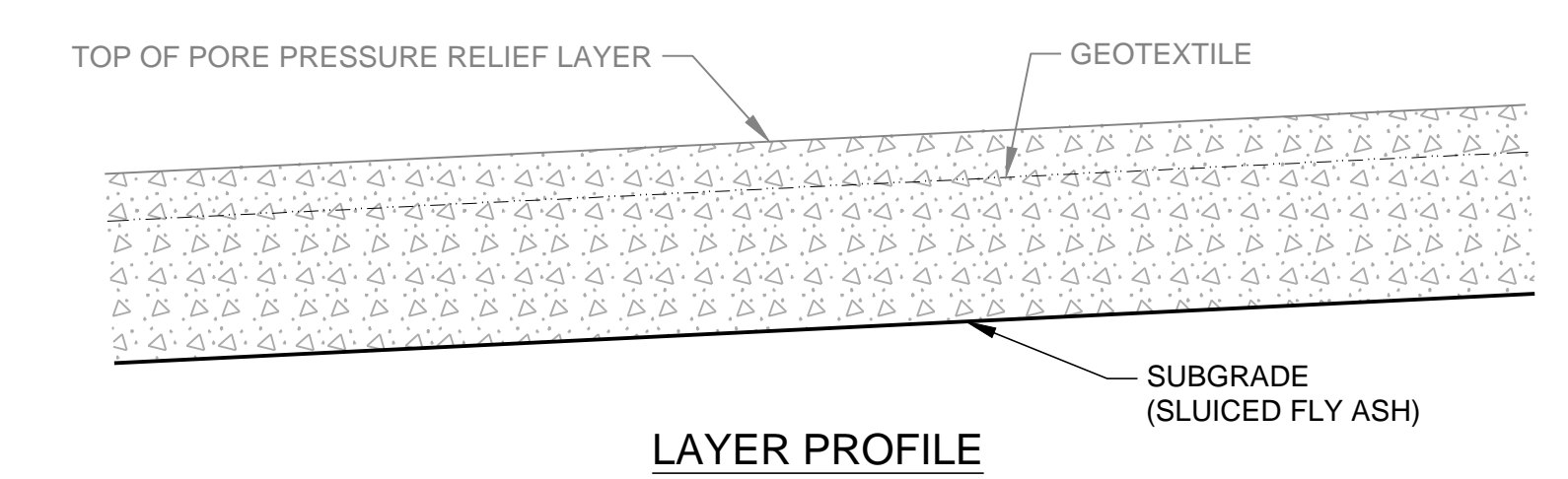
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RECORD DRAWING

0 75 150 300
SCALE FEET

- NOTE(S)**
1. ABANDONED PIPING WITHIN LIMITS OF SUBGRADE WAS REMOVED.
 2. CLAY COVER WITHIN LIMITS OF SUBGRADE WAS REMOVED.



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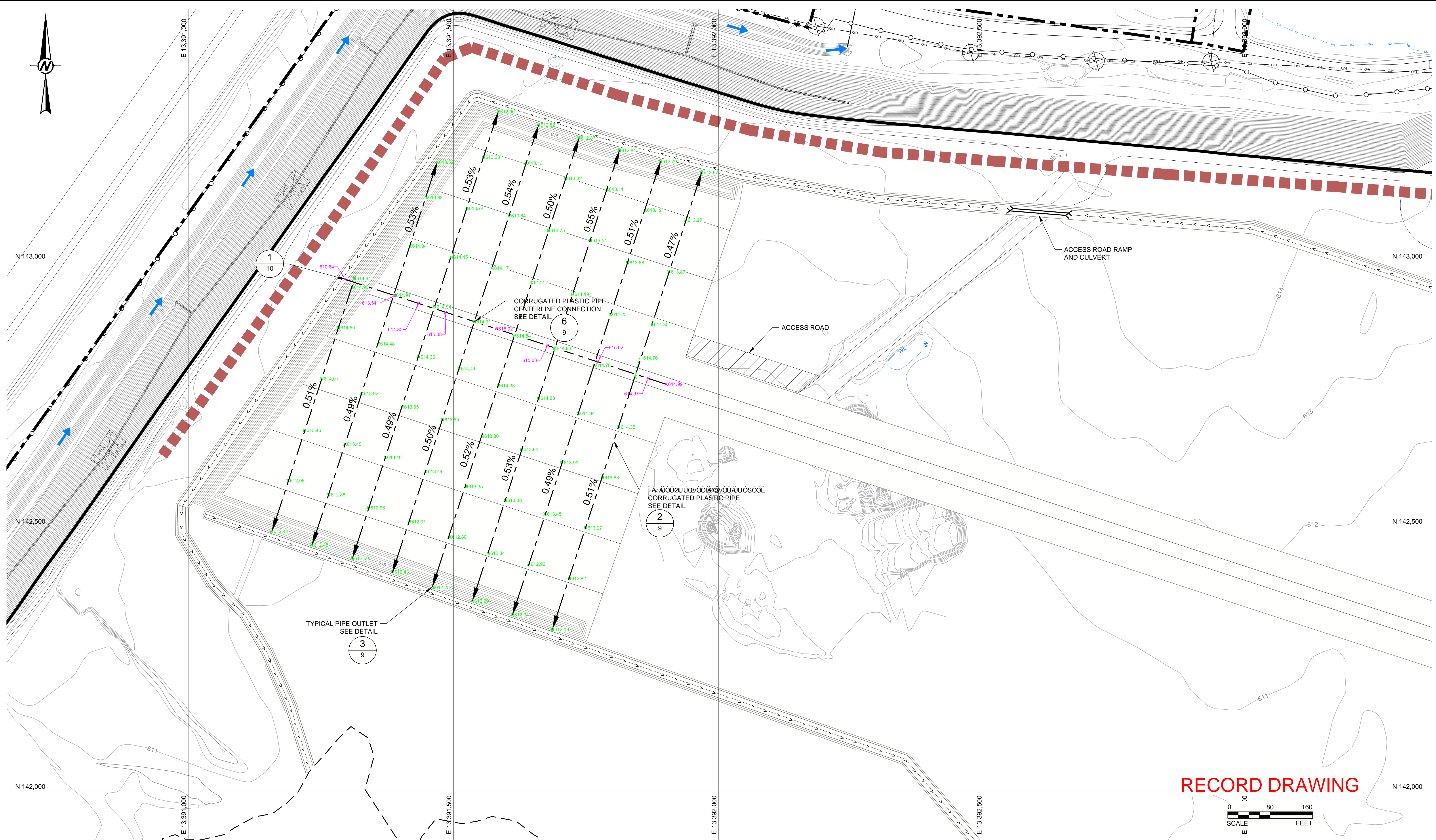
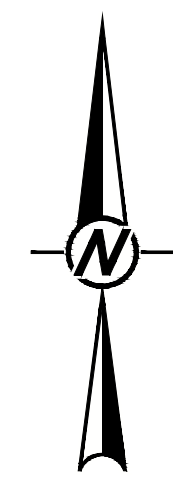
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DESIGN	JJS
REVIEW	JJS
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PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
RECORD TOP OF SUBGRADE AND SWALE PLAN

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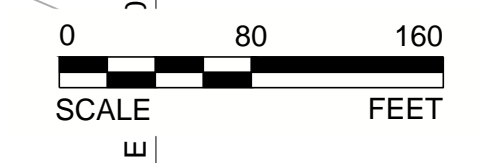


TYPICAL PIPE OUTLET
SEE DETAIL
3
9

CORRUGATED PLASTIC PIPE
CENTERLINE CONNECTION
SIZE DETAIL
6
9

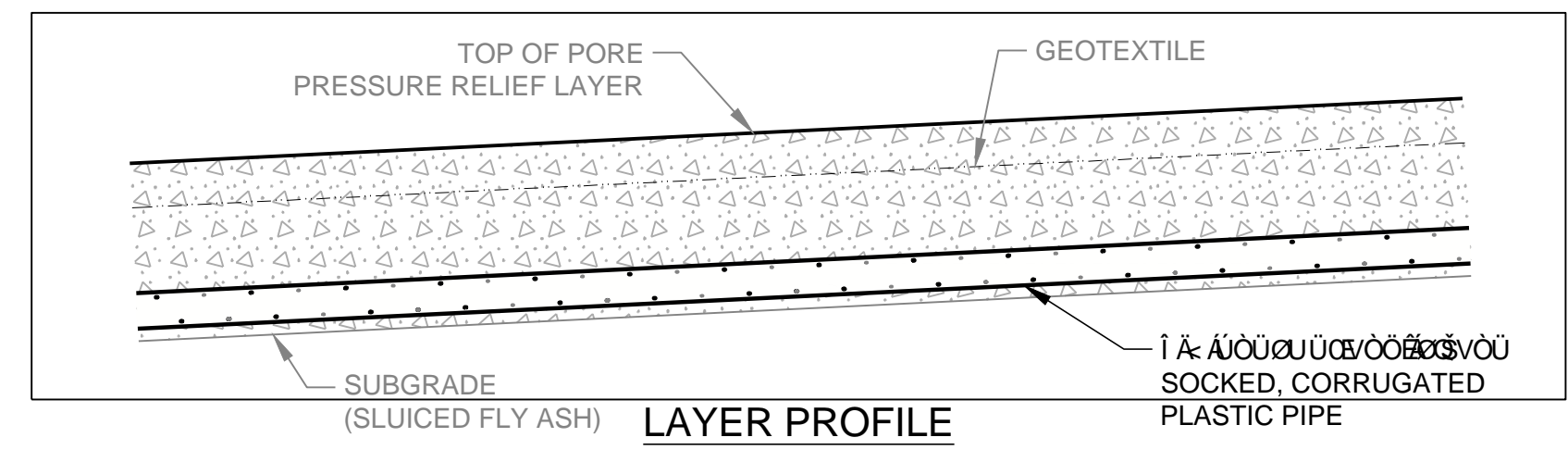
CORRUGATED PLASTIC PIPE
SEE DETAIL
2
9

RECORD DRAWING



NOTE(S)
1. RECORD POINTS PROVIDED BY KemTech, HOWEVER, CERTAIN POINTS WERE UPDATED BY EAGLE EXCAVATING USING A GPS SYSTEM, FOLLOWING ADDITIONAL FIELD WORK.

LEGEND
AS BUILT 6" PIPE ELEVATION
AS BUILT 8" PIPE ELEVATION



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PREPARED	JJS
DESIGN	JJS
REVIEW	JJS
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
RECORD PORE WATER RELIEF PIPING PLAN

PROJECT No.
1521809B

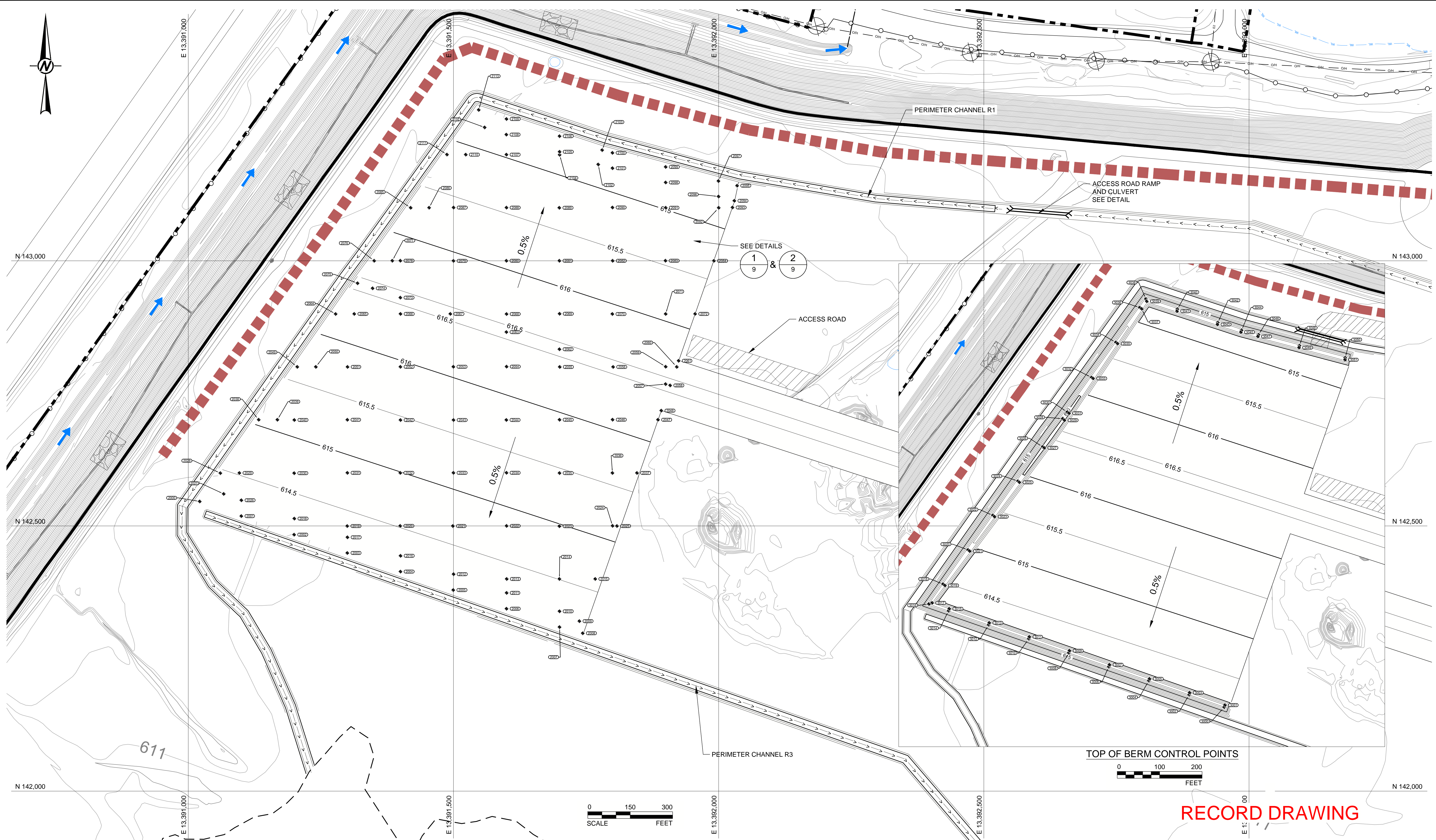
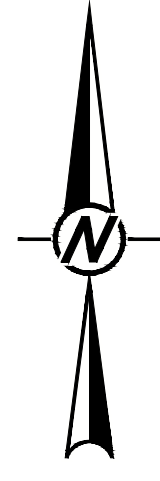
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Rev.

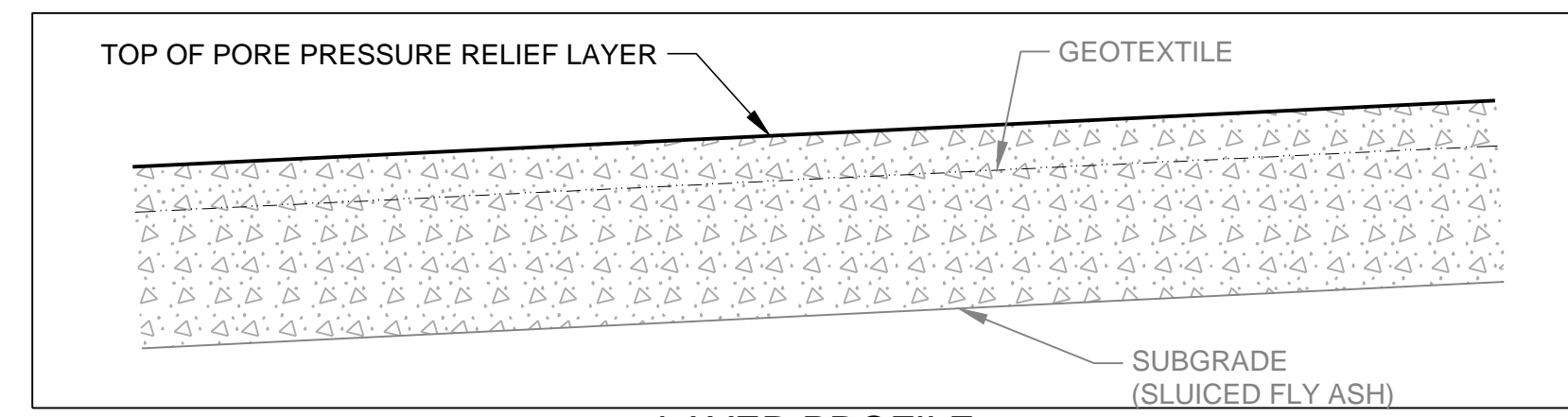
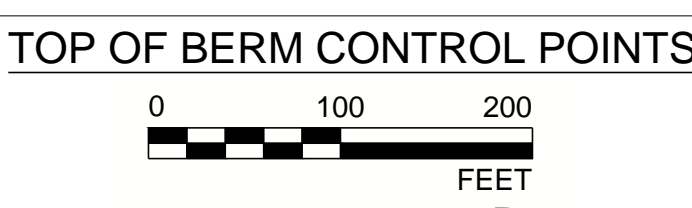
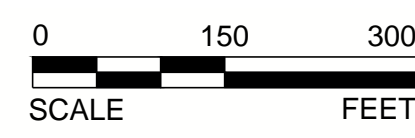
FIGURE
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SEE DETAILS
1 & 2
9 9



LAYER PROFILE

RECORD DRAWING

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MONROE, MI
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DESIGN	JJS
REVIEW	JJS
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
RECORD PORE WATER RELIEF SYSTEM AND BERMPAN

PROJECT No.
1521809B

CONTROL
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FIGURE
6

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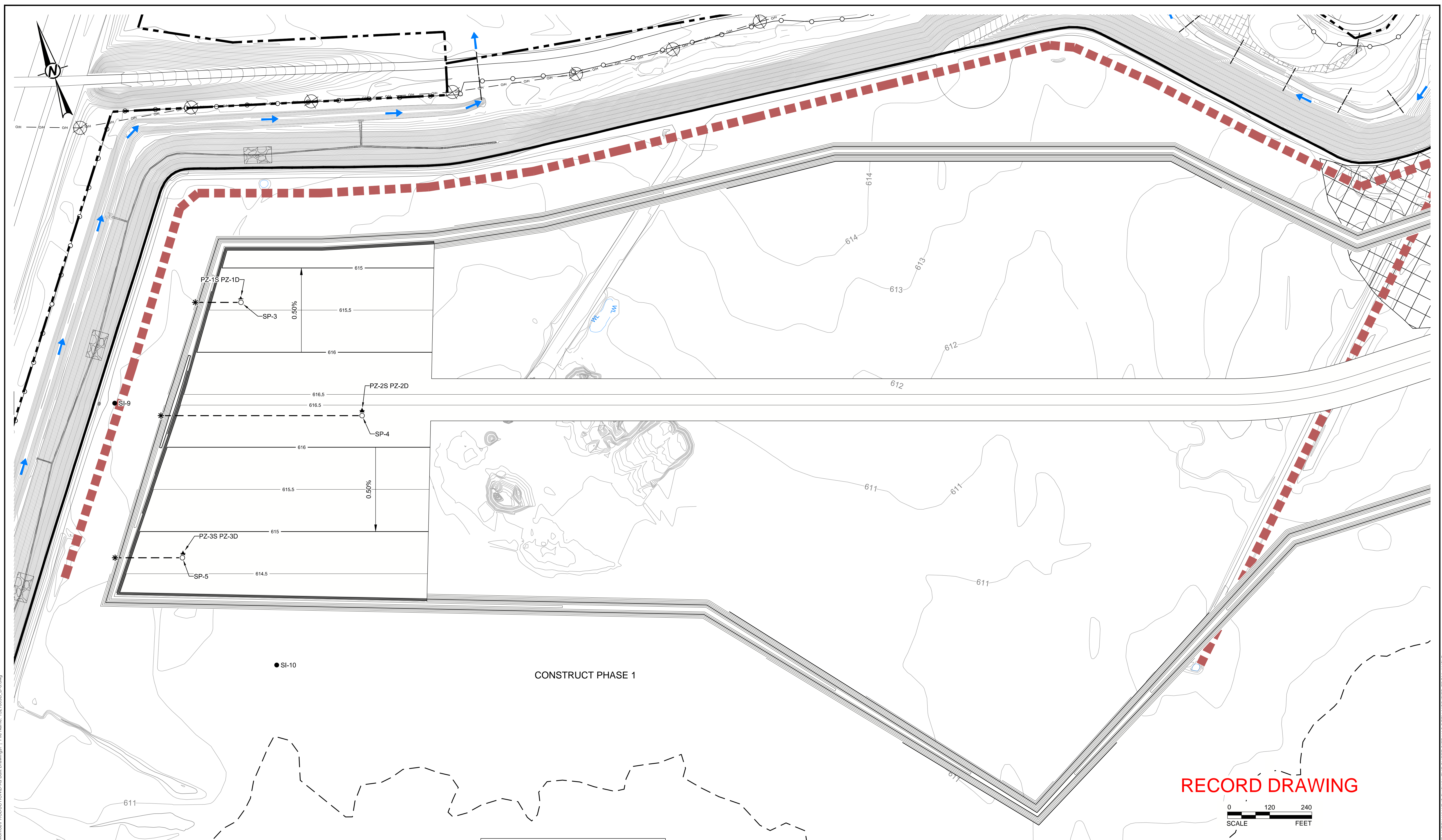
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2015 SUBGRADE CONSTRUCTION CONTROL POINTS

POINT NUMBER	NORTHING	EASTING	DESIGN ELEVATION	RECORD ELEVATION	Δ (ft) & A
1000	142,542.18	13,391,009.15	611.61	611.66	0.05
1001	142,510.58	13,391,100.12	611.60	611.52	-0.08
1002	142,475.44	13,391,200.04	611.59	611.01	-0.58
1003	142,440.88	13,391,300.16	611.58	611.54	-0.04
1004	142,400.13	13,391,416.70	611.57	611.59	0.03
1005	142,372.08	13,391,500.44	611.56	611.62	0.06
1006	142,335.77	13,391,599.85	611.55	611.09	-0.45
1007	142,300.86	13,391,699.60	611.54	611.54	0.01
1008	142,286.99	13,391,741.49	611.53	611.48	-0.05
1009	142,300.04	13,391,745.81	611.60	611.62	0.02
1010	142,399.75	13,391,499.91	611.70	611.65	-0.04
1011	142,400.22	13,391,599.98	611.85	611.85	0.00
1012	142,400.11	13,391,699.84	612.01	611.96	-0.05
1013	142,400.00	13,391,780.95	612.13	612.13	NOTE 2
1014	142,500.01	13,391,200.04	611.70	611.56	-0.15
1015	142,499.88	13,391,299.97	611.86	611.82	-0.04
1016	142,499.94	13,391,400.30	612.02	612.02	0.01
1017	142,500.19	13,391,499.46	612.17	612.23	0.06
1018	142,499.95	13,391,599.83	612.33	612.27	-0.06
1019	142,499.93	13,391,699.85	612.48	612.38	-0.10
1020	142,500.70	13,391,799.22	612.64	612.59	-0.05
1021	142,500.01	13,391,816.00	612.66	611.93	-0.73
1022	142,599.98	13,391,051.07	611.95	611.85	-0.09
1023	142,600.00	13,391,100.04	612.03	612.06	0.04
1024	142,599.83	13,391,200.02	612.18	612.12	-0.06
1025	142,599.94	13,391,299.97	612.34	612.27	-0.07
1026	142,599.82	13,391,399.96	612.49	612.48	-0.01
1027	142,599.91	13,391,499.39	612.65	612.64	0.00
1028	142,599.78	13,391,600.19	612.80	612.80	0.00
1029	142,600.14	13,391,700.04	612.96	612.92	-0.03
1030	142,600.08	13,391,800.07	613.11	612.96	-0.16
1031	142,600.08	13,391,851.09	613.19	611.96	-1.23
1032	142,699.95	13,391,123.36	612.54	612.53	-0.01
1033	142,699.82	13,391,200.05	612.66	612.56	-0.10
1034	142,700.58	13,391,300.16	612.81	612.71	-0.10
1035	142,699.99	13,391,399.95	612.97	612.94	-0.03
1036	142,699.97	13,391,500.14	613.12	613.05	-0.07
1037	142,700.17	13,391,600.21	613.28	613.28	0.00
1038	142,699.90	13,391,700.02	613.43	613.36	-0.07
1039	142,699.94	13,391,799.97	613.59	613.49	-0.10
1040	142,699.29	13,391,885.59	613.72	613.72	NOTE 2
1041	142,717.52	13,391,891.89	613.81	613.81	NOTE 2
1043	142,799.87	13,391,195.73	613.12	613.05	-0.07
1044	142,799.98	13,391,299.97	613.29	613.20	-0.08
1045	142,800.04	13,391,400.05	613.44	613.36	-0.08
1046	142,800.09	13,391,499.99	613.60	613.61	0.01
1047	142,800.01	13,391,600.01	613.75	613.69	-0.07
1048	142,799.78	13,391,699.86	613.91	613.90	-0.01
1049	142,799.47	13,391,800.90	614.06	614.05	-0.02
1050	142,787.54	13,391,899.82	614.06	614.06	NOTE 2
1051	142,800.06	13,391,900.02	613.91	613.91	NOTE 2
1052	142,811.93	13,391,924.96	613.81	613.81	NOTE 2
1053	142,799.70	13,391,962.18	613.81	613.81	NOTE 2
1054	142,900.02	13,391,268.15	613.71	613.07	-0.64
1055	142,899.45	13,391,300.58	613.76	613.79	0.03
1056	142,900.03	13,391,399.45	613.92	613.86	-0.06
1057	142,900.61	13,391,494.80	614.06	614.03	-0.04
1058	142,900.61	13,391,602.66	613.90	613.89	-0.01
1059	142,866.12	13,391,599.65	614.06	614.02	-0.05
1060	142,832.85	13,391,700.16	614.06	614.00	-0.07
1061	142,899.85	13,391,700.43	613.74	613.69	-0.06
1062	142,900.07	13,391,799.84	613.59	613.69	0.10
1063	142,900.58	13,391,899.94	613.43	613.48	0.05
1064	142,901.15	13,391,955.49	613.35	613.41	0.06
1065	142,931.20	13,391,400.87	614.06	614.02	-0.04
1066	142,960.25	13,391,310.61	614.06	614.04	-0.03
1067	142,998.74	13,391,340.72	613.83	613.85	0.02
1068	142,999.91	13,391,399.59	613.73	613.79	0.06
1069	142,999.39	13,391,500.01	613.58	613.54	-0.04
1070	142,999.20	13,391,601.45	613.42	613.43	0.01
1071	143,000.45	13,391,698.92	613.27	613.31	0.04
1072	142,999.79	13,391,800.24	613.11	613.11	0.00
1073	142,999.42	13,391,899.64	612.96	612.95	-0.01
1074	143,000.29	13,391,990.81	612.82	612.79	-0.03
1075	143,099.98	13,391,409.86	613.24	613.23	-0.01
1076	143,099.46	13,391,500.08	613.10	613.08	-0.02
1077	143,099.92	13,391,600.18	612.95	612.95	0.00
1078	143,100.13	13,391,701.08	612.79	612.82	0.03
1079	143,100.05	13,391,800.14	612.64	612.73	0.09
1080	143,100.22	13,391,900.88	612.48	612.53	0.04
1081	143,099.67	13,392,000.06	612.33	612.36	0.03
1082	143,100.20	13,392,025.55	612.29	612.28	-0.01
1083	143,113.21	13,392,030.22	612.22	612.20	-0.01
1084	143,200.32	13,391,478.96	612.66	612.72	0.06
1085	143,200.09	13,391,600.29	612.47	612.46	-0.02
1086	143,199.94	13,391,700.28	612.32	612.31	-0.01
1087	143,199.87	13,391,800.16	612.16	612.19	0.03
1088	143,199.88	13,391,843.46	612.10	612.16	0.06
1089	143,184.44	13,391,900.07	612.08	612.09	0.01
1090	143,158.76	13,392,000.31	612.05	612.03	-0.02
1091	143,147.70	13,392,034.95	612.04	612.04	0.00
1092	143,291.88	13,391,545.80	612.11	612.21	0.10
1093	143,275.58	13,391,600.00	612.11	612.11	NOTE 2
1094	143,243.10	13,391,700.09	612.11	612.11	NOTE 2
1095	143,216.53	13,391,782.68	612.11	612.07	-0.04

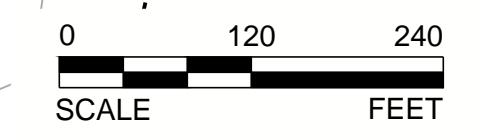
2015 TOP OF PORE PRESSURE RELIEF LAYER CONSTRUCTION CONTROL POINTS

POINT NUMBER	NORTHING	EASTING	DESIGN ELEVATION	Record Elevation	Δ (ft) & A
2000	142,546.20	13,391,021.66	614.15	614.10	-0.05
2001	142,516.60	13,391,100.06	614.14	614.15	0.02
2002	142,483.57	13,391,199.65	614.13	614.20	0.07
2003	142,449.03	13,391,299.92	614.12	614.14	0.02
2004	142,413.90	13,391,400.17	614.11	614.07	-0.04
2005	142,379.14	13,391,499.65	614.09	614.13	0.04
2006	142,343.88	13,391,600.04	614.08	614.06	-0.03
2007	142,309.32	13,391,700.02	614.07	614.01	-0.06
2008	142,297.72	13,391,743.78	614.07	614.05	-0.02
2009	142,320.49	13,391,737.43	614.21	614.23	0.02
2010	142,339.07	13,391,700.00	614.22	614.25	0.03
2011	142,373.75	13,391,600.26	614.23	614.23	0.01
2012	142,409.06	13,391,500.25	614.24	614.23	-0.01
2013	142,399.99	13,391,600.45	614.35	614.33	-0.02
2014	142,400.02	13,391,699.82	614.51	614.46	-0.05
2015	142,400.03	13,391,767.34	614.63	614.60	-0.03
2016	142,443.68	13,391,400.28	614.25	614.27	0.02
2017	142,478.61	13,391,300.18	614.26	614.26	0.00
2018	142,513.84	13,391,200.04	614.27	614.31	0.04
2019	142,499.82	13,391,299.81	614.36	614.32	-0.04
2020	142,500.09	13,391,399.68	614.52	614.57	0.06
2021	142,499.86	13,391,499.86	614.67	614.66	-0.01
2022	142,500.05	13,391,600.18	614.83	614.89	0.06
2023	142,499.56	13,391,699.77	614.98	614.91	-0.07
2024	142,500.18	13,391,800.00	615.14	615.17	0.03
2025	142,499.85	13,391,808.22	615.16	615.20	0.04
2026	142,548.48	13,391,100.19	614.28	614.26	-0.03
2027	142,560.42	13,391,066.90	614.28	614.22	-0.07
2028	142,599.60	13,391,060.56	614.46	614.45	-0.02
2029	142,599.89	13,391,096.41	614.52	614.53	0.01
2030	142,599.55	13,391,199.77	614.68	614.68	0.00
2031	142,600.12	13,391,300.08	614.84	614.88	0.05
2032	142,600.07	13,391,399.80	614.99	614.98	-0.01
2033	142,600.13	13,391,499.98	615.15	615.13	-0.02
2034	142,600.23	13,391,600.00	615.30	615.25	-0.05
2035	142,599.73	13,391,699.99	615.46	615.45	-0.01
2036	142,599.91	13,391,799.74	615.61	615.56	-0.05
2037	142,599.95	13,391,846.04	615.69	615.68	-0.01
2038	142,700.32	13,391,132.82	615.05	615.03	-0.02
2039	142,700.27	13,391,168.10	615.11	615.08	-0.02
2040	142,699.80	13,391,200.05	615.16	615.15	-0.01
2041	142,700.16	13,391,300.01	615.31	615.36	0.05
2042	142,699.69	13,391,400.13	615.47	615.44	-0.03
2043	142,699.74	13,391,500.15	615.62	615.68	0.06
2044	142,699.83	13,391,599.77	615.78	615.79	0.01
2045	142,700.24	13,391,700.02	615.93	615.95	0.02
2046	142,700.36	13,391,800.08	616.09	616.03	-0.06
2047	142,700.22	13,391,885.64	616.22	616.20	-0.02
2048	142,717.46	13,391,891.88	616.31	616.29	-0.02
2049	142,799.95	13,391,205.54	615.64	615.59	-0.05
2050	142,799.95	13,391,240.57	615.69	615.71	0.01
2051	142,799.88	13,391,300.34	615.79	615.82	0.04
2052	142,800.07	13,391,400.07	615.94	615.91	-0.03
2053	142,800.17	13,391,499.87	616.10	616.06	-0.04
2054	142,800.41	13,391,600.20	616.25	616.26	0.01
2055	142,799.82	13,391,700.37	616.41	616.45	0.04
2056	142,799.85	13,391,801.07	616.56	616.57	0.01
2057	142,787.53	13,391,900.05	616.56	616.58	0.02
2058	142,765.02	13,391,908.55	616.56	616.60	0.03
2059	142,800.31	13,391,900.15	616.		



CONSTRUCT PHASE 1

RECORD DRAWING



- LEGEND**
- SI-1 SLOPE INCLINOMETER
 - ▲ PZ-1D
▲ PZ-1S VIBRATING WIRE PIEZOMETER (VWP) PAIR (DEEP & SHALLOW)
 - SP-1 SETTLEMENT PLATE
 - * REMOTE (CABLED) DATA LOGGER LOCATION FOR VWP
 - DATA LOGGER CABLE

RECORD INSTRUMENTATION INFORMATION			
INSTRUMENT IDENTIFICATION	NORTHING	EASTING	ELEVATION
PZ-1S PZ-1D	143,132.58	13,391,553.77	614.71
PZ-2S PZ-2D	142,791.36	13,391,744.99	615.92
PZ-3S PZ-3D	142,601.23	13,391,236.06	614.26
SI-10	142,280.24	13,391,364.86	612.45
SI-9	142,989.72	13,391,192.71	611.28
SP-3	143,124.35	13,391,551.76	615.34
SP-4	142,779.81	13,391,741.28	616.31
SP-5	142,591.55	13,391,231.59	614.76

CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI



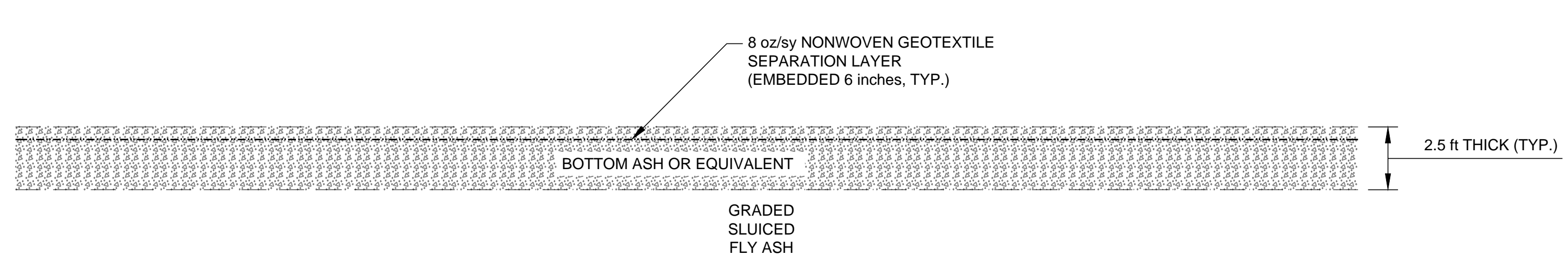
YYYY-MM-DD	2015-09-10
PREPARED	JJS
DESIGN	JJS
REVIEW	JJS
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

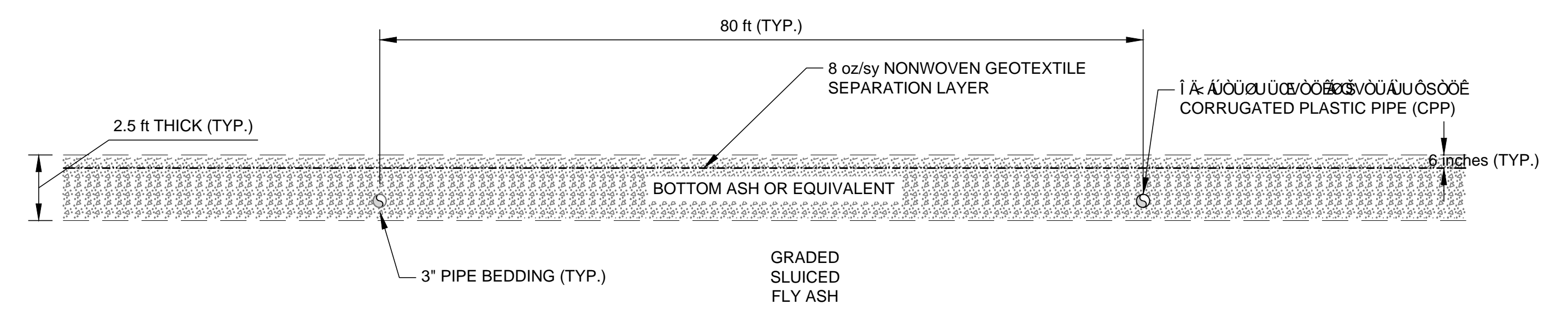
TITLE
RECORD MONITORING LOCATIONS FOR PHASE 1

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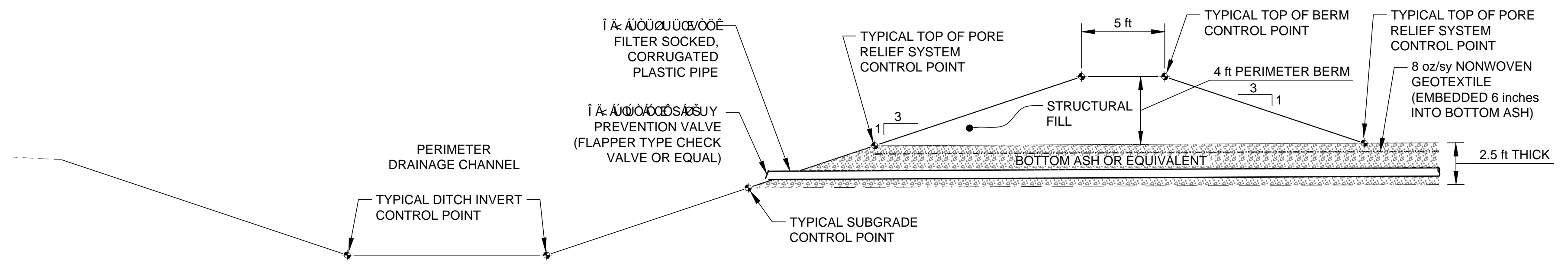
IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D



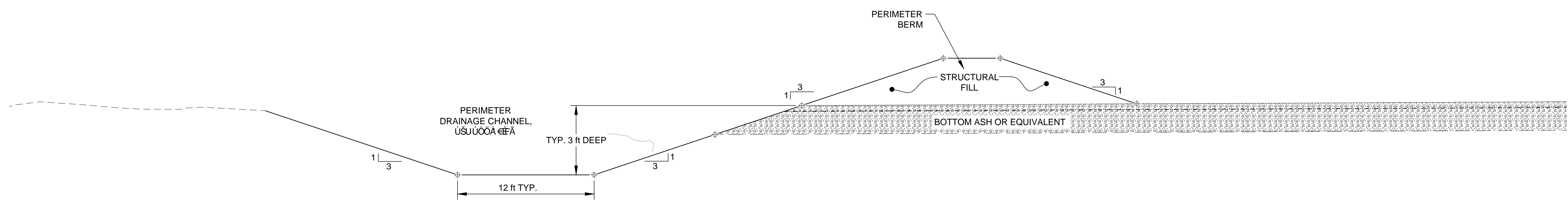
1 PORE RELIEF SYSTEM BASE LINER
9



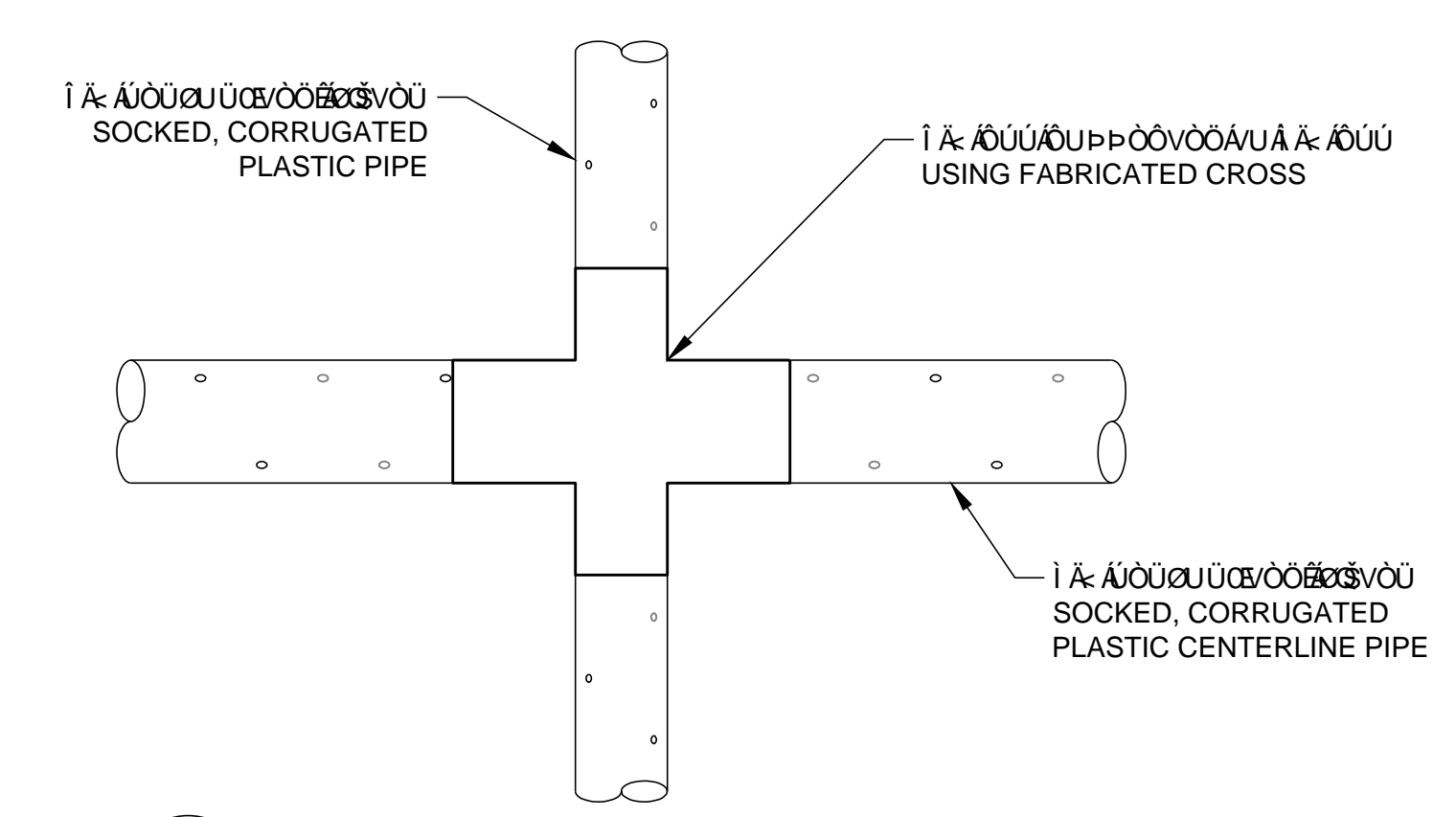
2 PORE RELIEF SYSTEM BASE LINER w/PIPES
9



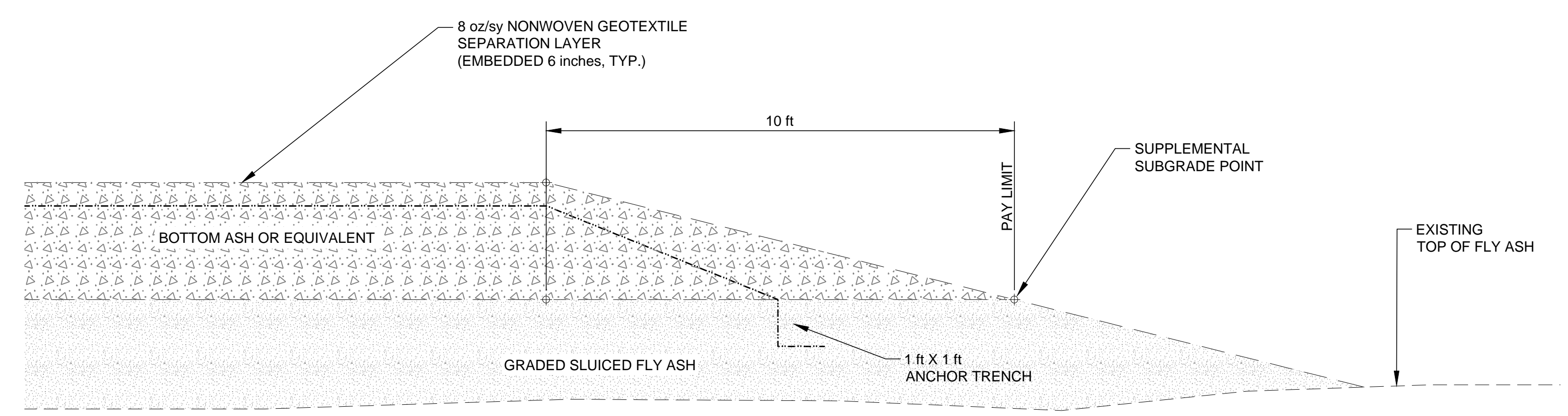
3 PERIMETER BERM WITH NORTH-SOUTH PORE RELIEF PIPING SHOWN
9



4 PERIMETER BERM WITHOUT PIPING
9



6 CORRUGATED PLASTIC PIPE CENTERLINE CONNECTION
9



5 TYPICAL LEADING EDGE DETAIL
9

RECORD DRAWING

CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

CONSULTANT	YYYY-MM-DD	2015-09-10
	PREPARED	JJS
	DESIGN	JJS
	REVIEW	JJS
	APPROVED	DML

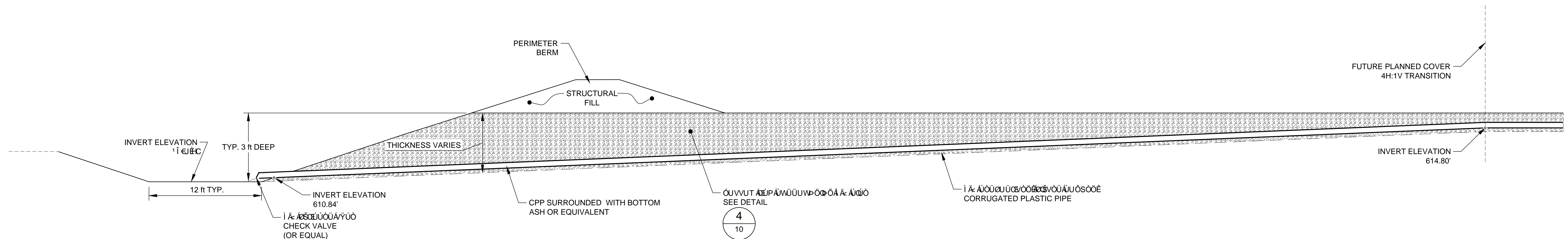
TITLE
GENERAL DETAILS
SHEET 1

PROJECT No.	CONTROL	Rev.	FIGURE
1521809B	A		9

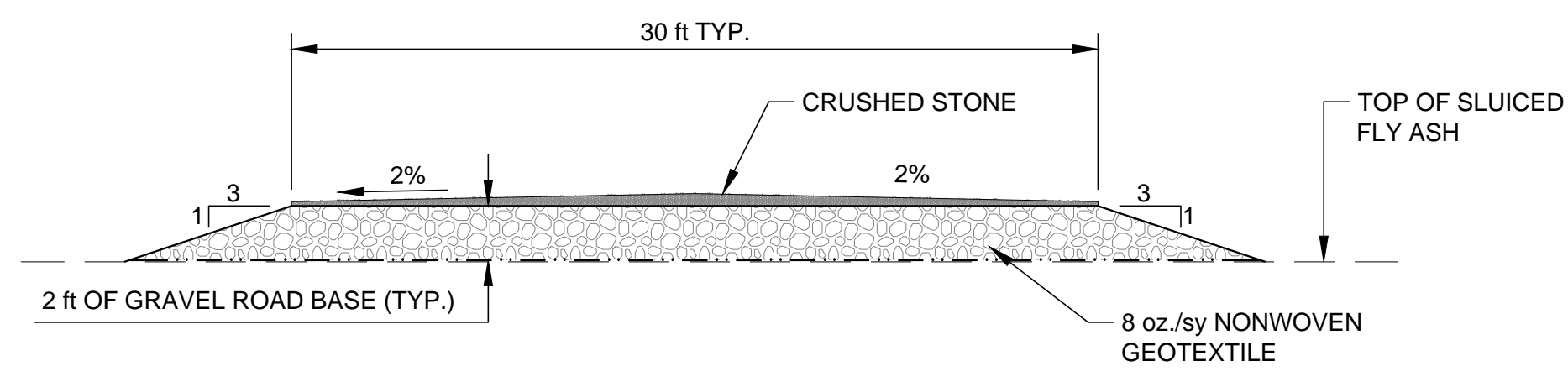


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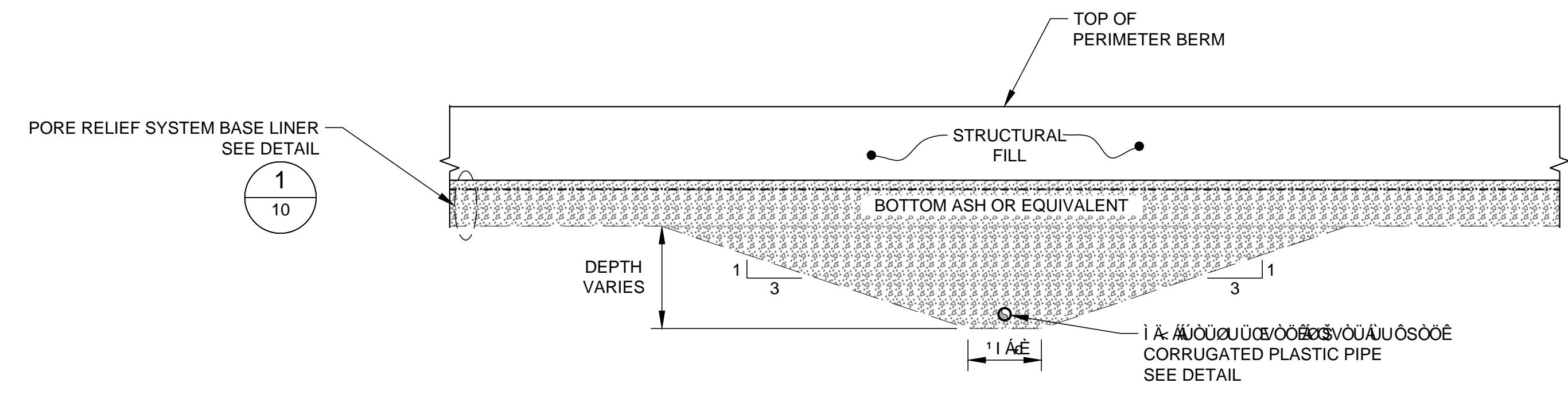
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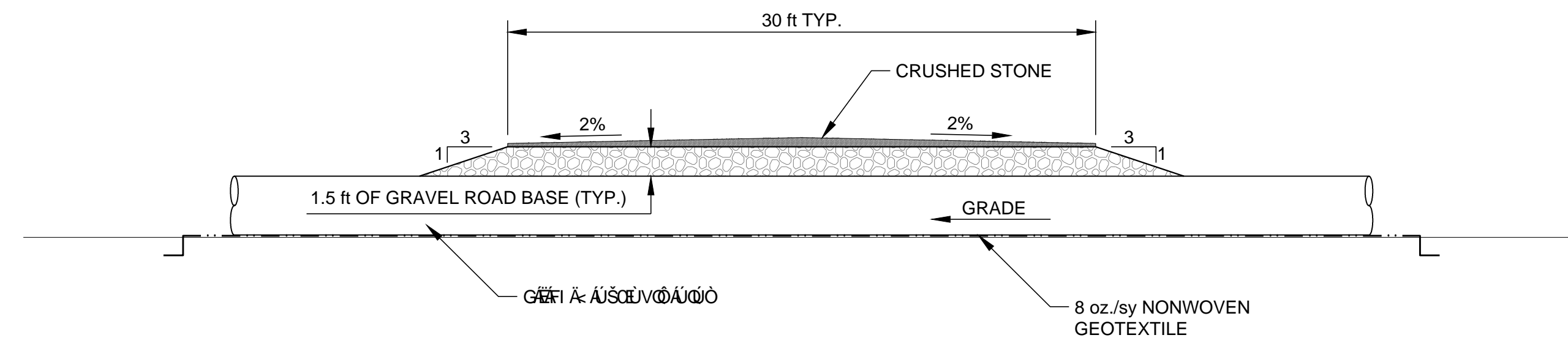
1 PERIMETER BERM WITH WESTERN CENTRAL PORE PRESSURE RELIEF PIPE SHOWN



2 TYPICAL NEW ROAD SECTION



4 PIPE TRENCH PAST 4H:1V SLOPE



3 TEMPORARY ACCESS ROAD RAMP AND CULVERT DETAIL

RECORD DRAWING

CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

CONSULTANT



YYYY-MM-DD 2015-09-10
PREPARED JJS
DESIGN JJS
REVIEW JJS
APPROVED DML

PROJECT
MONROE POWER PLANT ASH BASIN
2015 PHASE 1 RECORD DRAWINGS

TITLE
GENERAL DETAILS
SHEET 2

PROJECT No.
1521809B

CONTROL
A

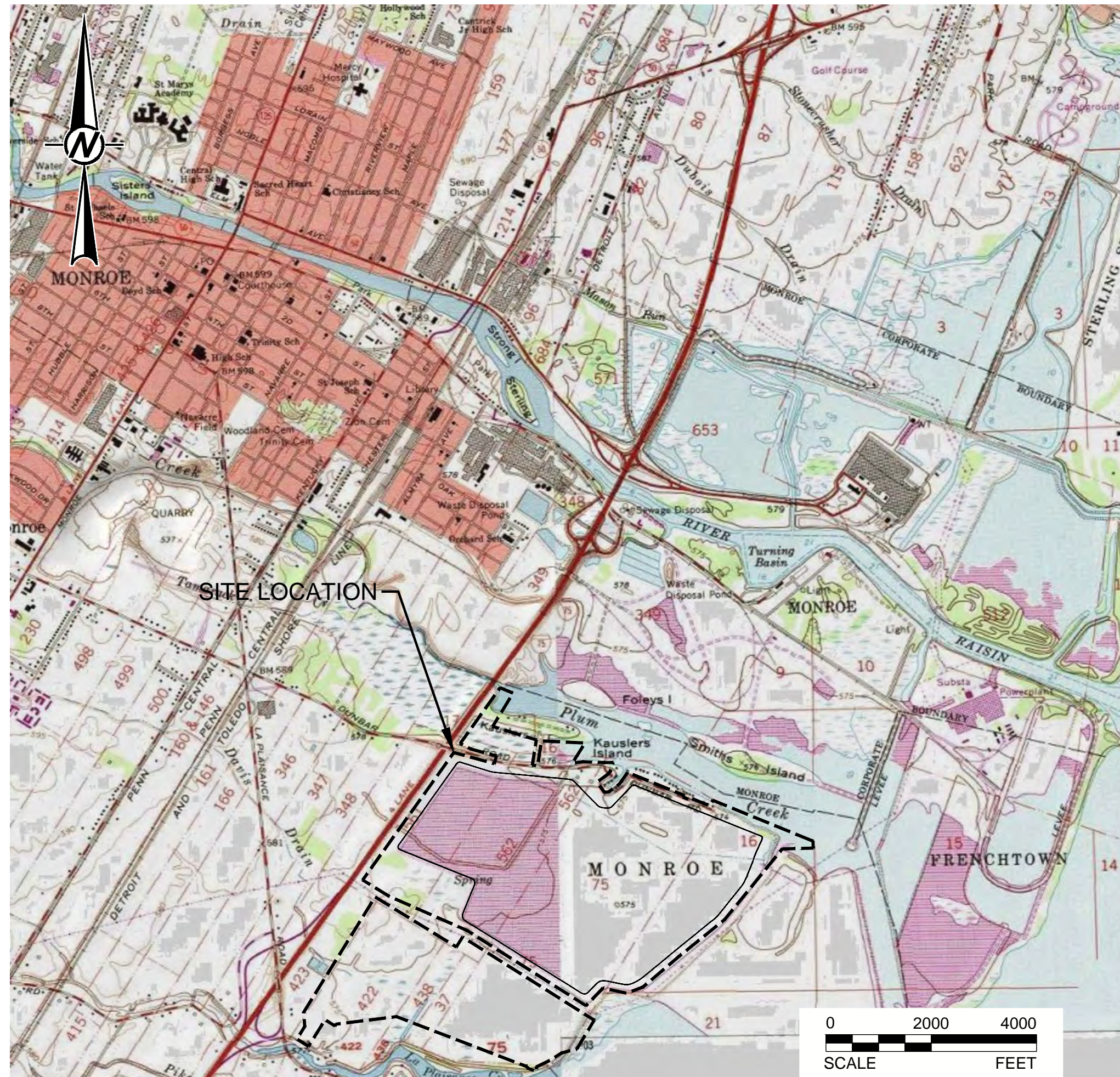
Rev.

FIGURE
10

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IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D

DTE ENERGY MONROE FLY ASH BASIN 2017 PHASE 2 RECORD DRAWINGS



Prepared for:
DTE ENERGY
One Energy Plaza
Detroit, MI 48226

Prepared by:
Golder Associates Inc.
15851 S. US 27 Suite 50
Lansing, Michigan USA 48906

INDEX OF DRAWINGS			
Sheet Number	Drawing Title	Current Revision	Date
1	Title Sheet	0	11/20/2017
2	Legend, References and General Notes	0	11/20/2017
3	Site Plan	0	11/20/2017
4	Record Top of Subgrade Plan	0	11/20/2017
5	Record Pore Water Relief Piping Plan	0	11/20/2017
6	Record Pore Water Relief System Plan	0	11/20/2017
7	Record Top of Berm Plan	0	11/20/2017
8	Record Top of Subgrade Construction Control / QA/QC Points	0	11/20/2017
9	Record Pore Water Relief System Construction Control / QA/QC Points	0	11/20/2017
10	Record Monitoring Locations for Phase 2	0	11/20/2017
11	General Details - Sheet 1	0	11/20/2017
12	General Details - Sheet 2	0	11/20/2017

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CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

CONSULTANT



YYYY-MM-DD 2017-11-20
PREPARED JJS
DESIGN JJS
REVIEW SEF
APPROVED DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

TITLE
TITLE SHEET

PROJECT No. 1521809D CONTROL 1521809DA001.dwg

Rev. 0

FIGURE 1

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D

LEGEND

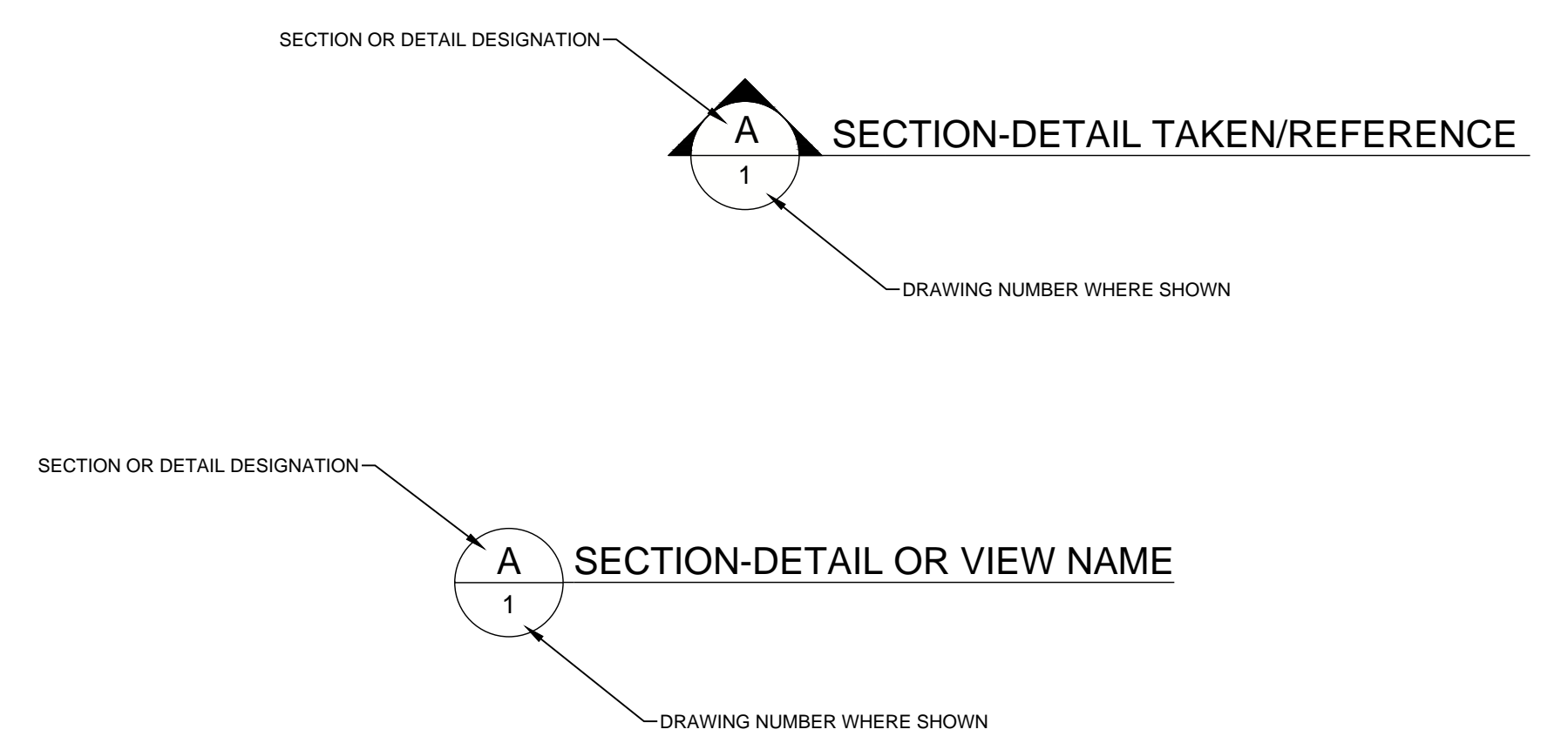
	PROPERTY BOUNDARY
	ASH BASIN EXISTING TOPOGRAPHY (1' AND 5' CONTOURS)
	WATER BOUNDARY
	FENCE LINE
	ELECTRIC TOWER
	ELECTRIC POLE
	UNDERGROUND ELECTRIC LINE
	CONSUMERS ENERGY ELECTRIC LINE
	ITC ELECTRIC LINE
	RIGHT OF WAY
	DRAINAGE DIRECTION
	SAMPLE LOCATION
	DECOMMISSIONED SLURRY PIPELINES
	ACTIVE SLURRY PIPELINES
	WETLAND
	STRUCTURE (RESIDENCE, BUSINESS)
	CULVERT
	BENCHMARK LOCATION
	WATER LINE
	VEHICLE CROSSING
	PROPOSED DRAINAGE CHANNEL
	PROPOSED PERFORATED, FILTER SOCKED, CORRUGATED PLASTIC PIPE (6" OR 8"Ø)
	AREA OF CLAY COVER
	AREA OF FILL TICK MARK
	AREA OF CUT TICK MARK
	CONTROL POINT
	CULVERT

MATERIALS LEGEND

	SLUICED FLY ASH
	BOTTOM ASH OR EQUIVALENT
	8 oz/sy GEOTEXTILE
	LOW PERMEABILITY SOILS COVER MATERIAL
	STRUCTURAL FILL
	TOPSOIL

- GENERAL NOTES**
- EXISTING TOPOGRAPHY IS AN AMALGAM OF GRADES FROM 2009 BASE MAP (DRAWING 6SE-0695-070-REV(B)) AND AS-BUILT GRADES FROM MITIGATED SECTIONS OF THE EMBANKMENT, PROVIDED BY DTE, DRAWING CREATED BY GEOSYNTEC CONSULTANTS, DATED 02/08/2013.
 - HORIZONTAL GRID COORDINATE SYSTEM UNITS ARE IN FEET AND REFERENCED TO MICHIGAN SOUTH STATE PLANE COORDINATES SYSTEM (NAD83). ELEVATIONS ARE IN FEET AND REFERENCED TO NATIONAL GEODETIC VERTICAL DATUM (NGVD29). NGVD29 DATUM IS 1.47 FT HIGHER THAN THE PLANT DATUM.
 - AREA OF CLAY COVER ADAPTED FROM "ATTACHMENT A" OF THE DTE ELECTRIC COMPANY SOLID WASTE OPERATING LICENSE APPLICATION, DATED DECEMBER 3, 2014.
 - PHASE I AND PERIMETER DRAINAGE SYSTEM WAS CONSTRUCTED, CERTIFIED AND APPROVED BY MDEQ IN 2015.

SECTION, DETAIL AND VIEW DESIGNATIONS



CLIENT
DTE ENERGY
MONROE POWER PLANT
MONROE, MI

CONSULTANT	YYYY-MM-DD	2017-11-20
	PREPARED	JJS
	DESIGN	JJS
	REVIEW	SEF
	APPROVED	DML



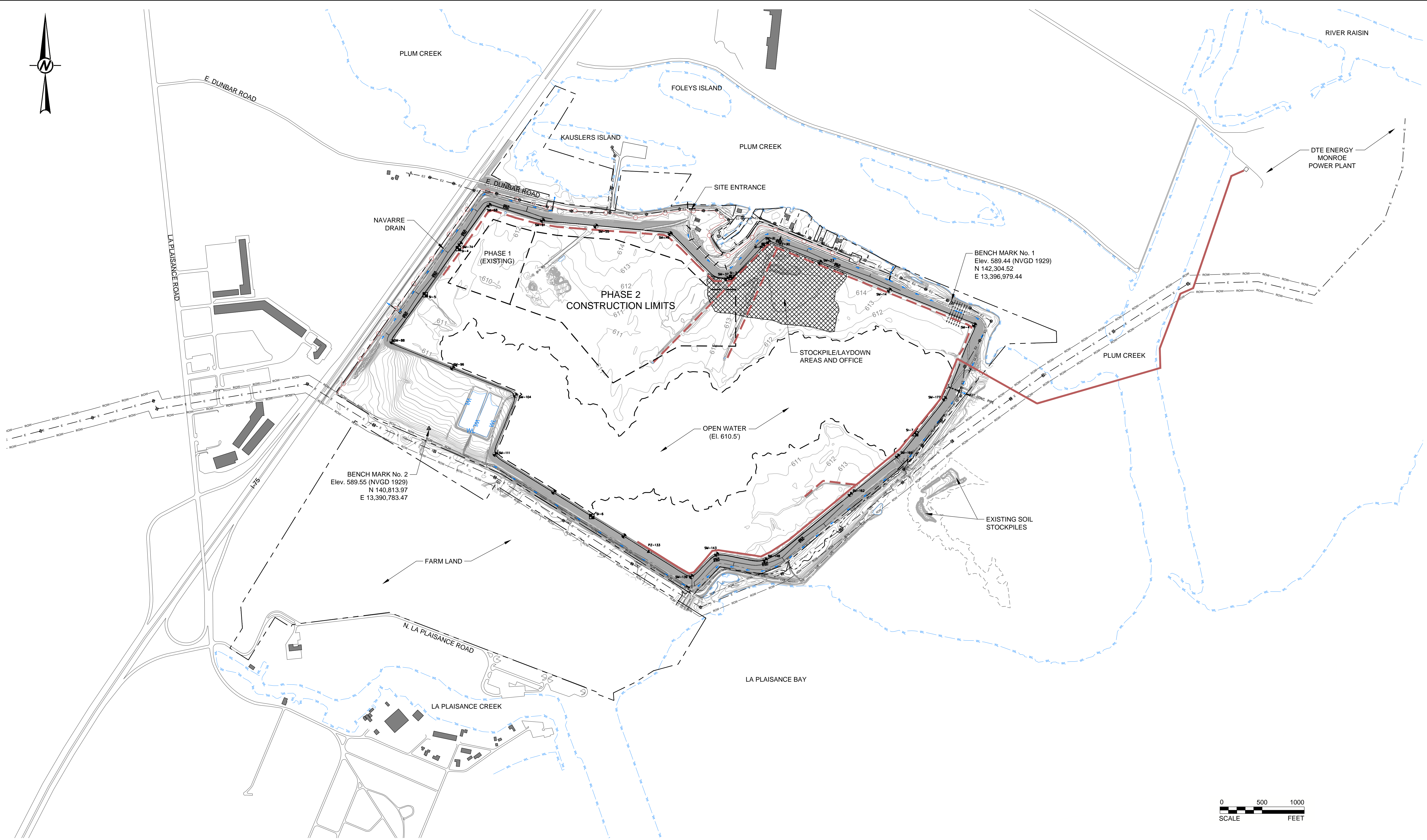
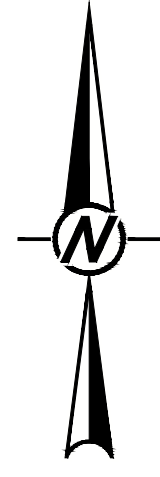
PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

TITLE
LEGEND, REFERENCES AND GENERAL NOTES

PROJECT No.	CONTROL	Rev.	FIGURE
1521809D	1521809DA002.dwg	0	2

Path: \\laning\cad\Projects\1521809D_DTE_Monroe_2016\PRODUCTION\A-PHASE 2 AS-BUILT... File Name: 1521809DA002.dwg

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D



Path: \\laning\cad\Projects\1521809D_DTE_Monroe_2016\PRODUCTION\A-PHASE 2 AS-BUILT_1 File Name: 1521809DA003.dwg

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PREPARED JJS
DESIGN JJS
REVIEW SEF
APPROVED DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

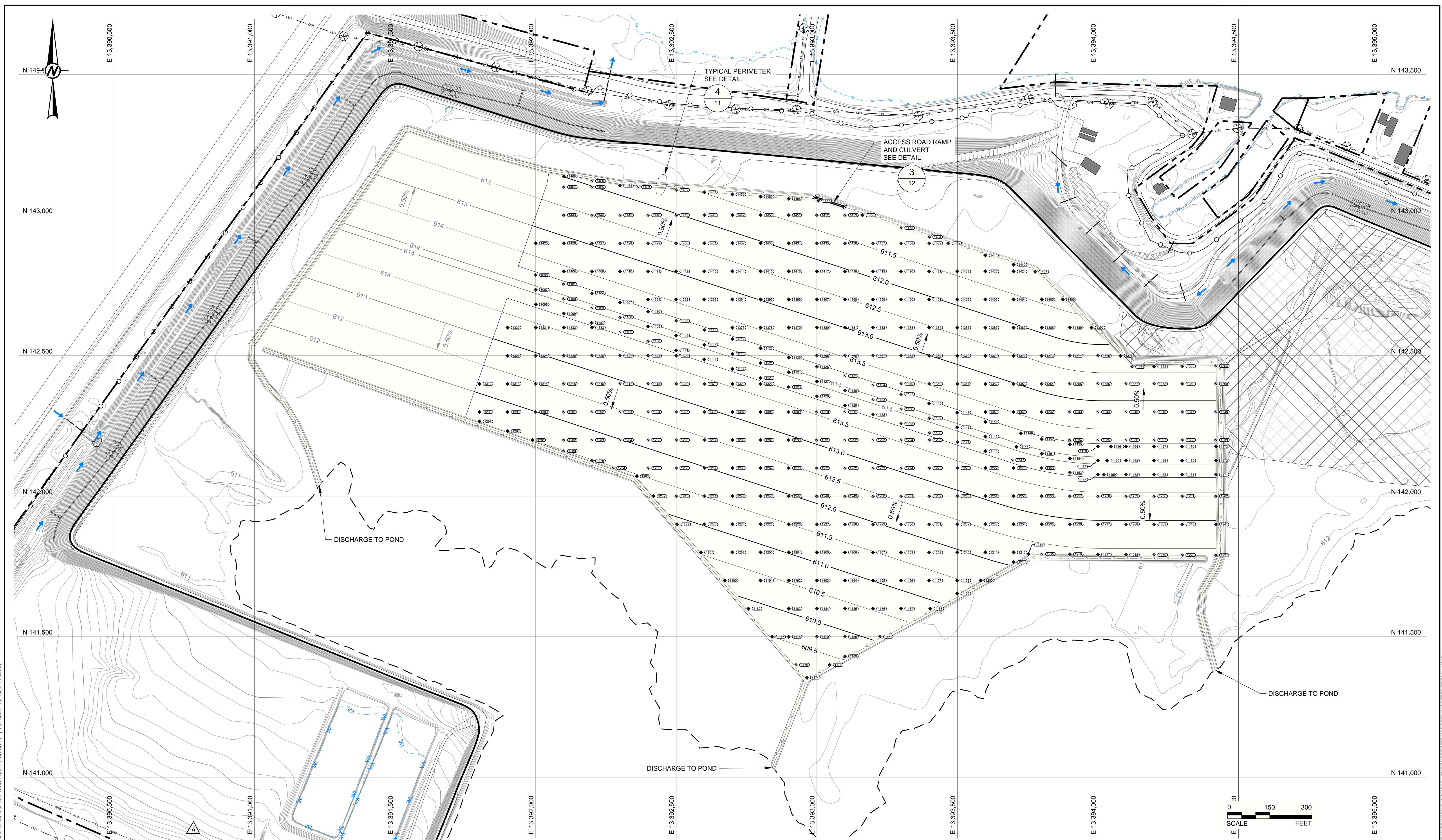
TITLE
SITE PLAN

PROJECT No. 1521809D CONTROL 1521809DA003.dwg

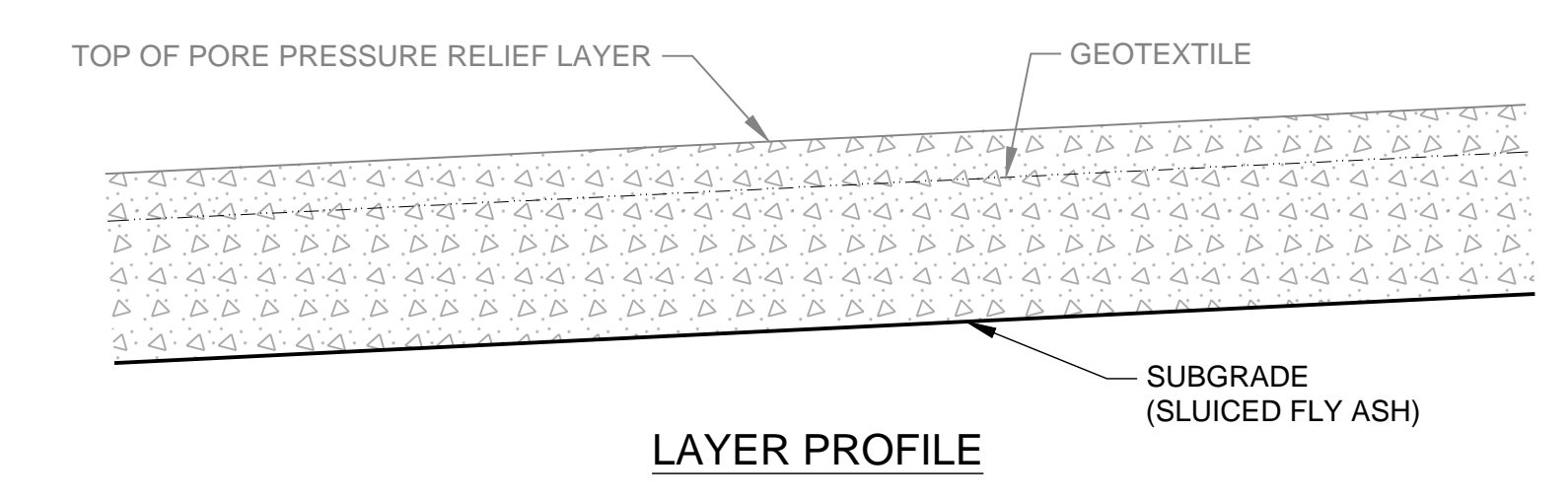
Rev. 0

FIGURE 3

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS/D



- NOTE(S)**
1. ABANDONED PIPING WITHIN LIMITS OF SUBGRADE WAS REMOVED.
 2. CLAY COVER WITHIN LIMITS OF SUBGRADE WAS REMOVED.



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DESIGN	JJS
REVIEW	SEF
APPROVED	DML

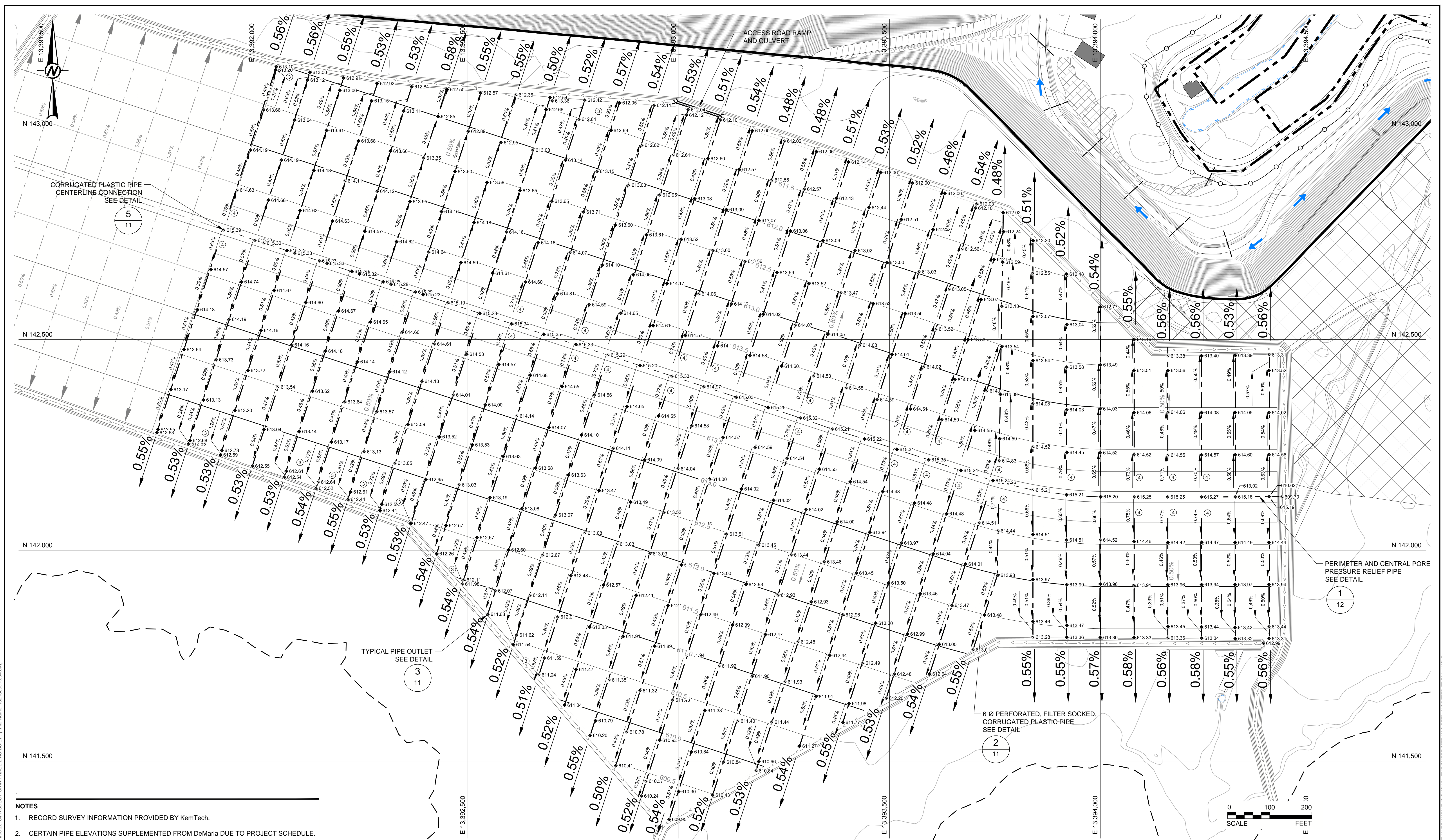
PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

TITLE
RECORD TOP OF SUBGRADE PLAN

PROJECT No.	CONTROL	Rev.	FIGURE
1521809D	1521809DA004.dwg	0	4

Path: \\laning\cad\Projects\1521809D - DTE Monroe 2016 PRODUCTION\PHASE 2 AS-BUILT - File Name: 1521809DA004.dwg

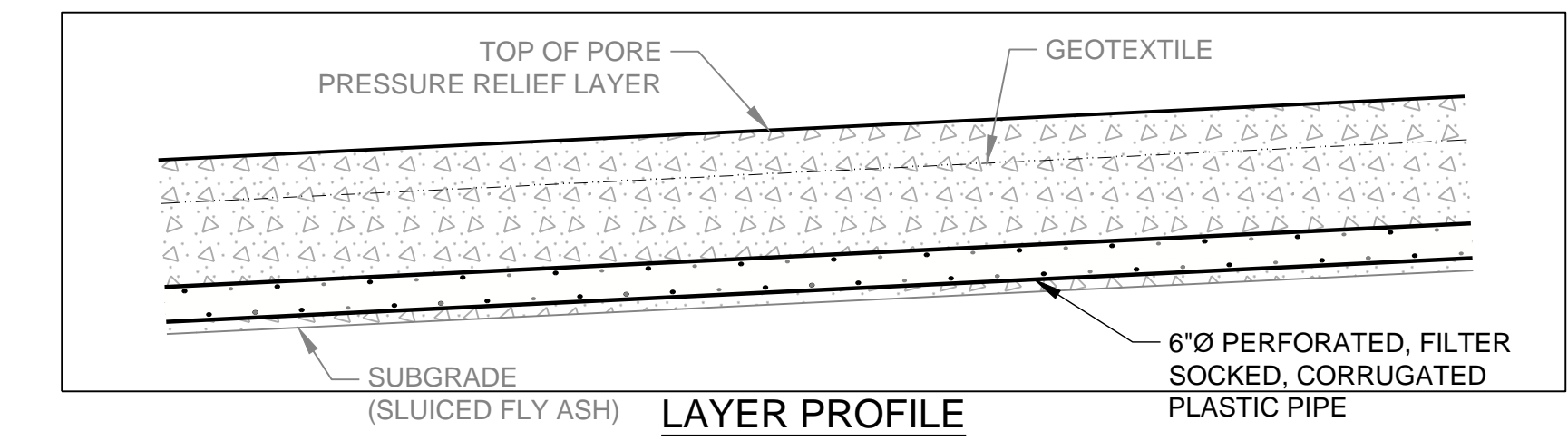
IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI D



- NOTES**
1. RECORD SURVEY INFORMATION PROVIDED BY KemTech.
 2. CERTAIN PIPE ELEVATIONS SUPPLEMENTED FROM DeMaria DUE TO PROJECT SCHEDULE.
 3. SMALL SEGMENTS OF PIPING NEAREST TO THE PERIMETER DITCH ALLOWED TO REMAIN SLIGHTLY STEEPER THAN DESIGN TO PROMOTE DRAINAGE AND DUE TO SMALL ELEVATION CHANGE TO NEXT UPSLOPE MEASUREMENT.
 4. INCREMENTAL SLOPE ON FIRST SEGMENT OFF OF CENTERLINE INCLUDES ADJUSTMENT FROM 8-inch TO 6-inch PIPE DIAMETER. INCREMENTAL SLOPE FROM CENTERLINE (8 inch PIPE) TO LATERAL (6-inch PIPE) INCLUDES DIAMETER CHANGE. ACCORDINGLY, SOME SEGMENTS WERE ALLOWED TO REMAIN SLIGHTLY STEEPER THAN DESIGN.

LEGEND

614.96	AS BUILT 6" PIPE ELEVATION
614.96	AS BUILT 8" PIPE ELEVATION



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MONROE, MI

CONSULTANT
Golder Associates

DATE	2017-11-20
PREPARED	JJS
DESIGN	JJS
REVIEW	SEF
APPROVED	DML

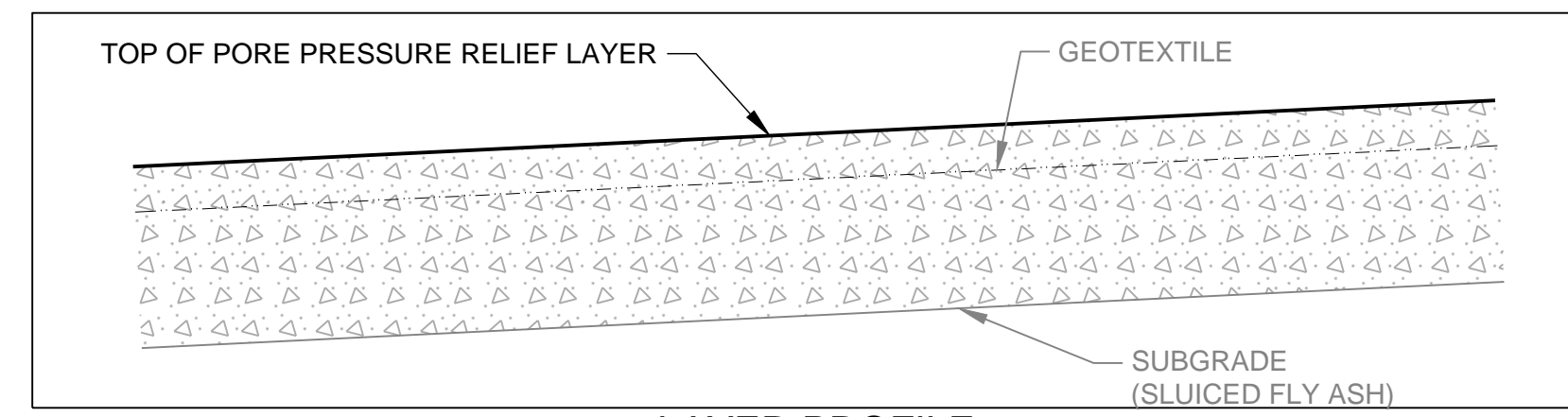
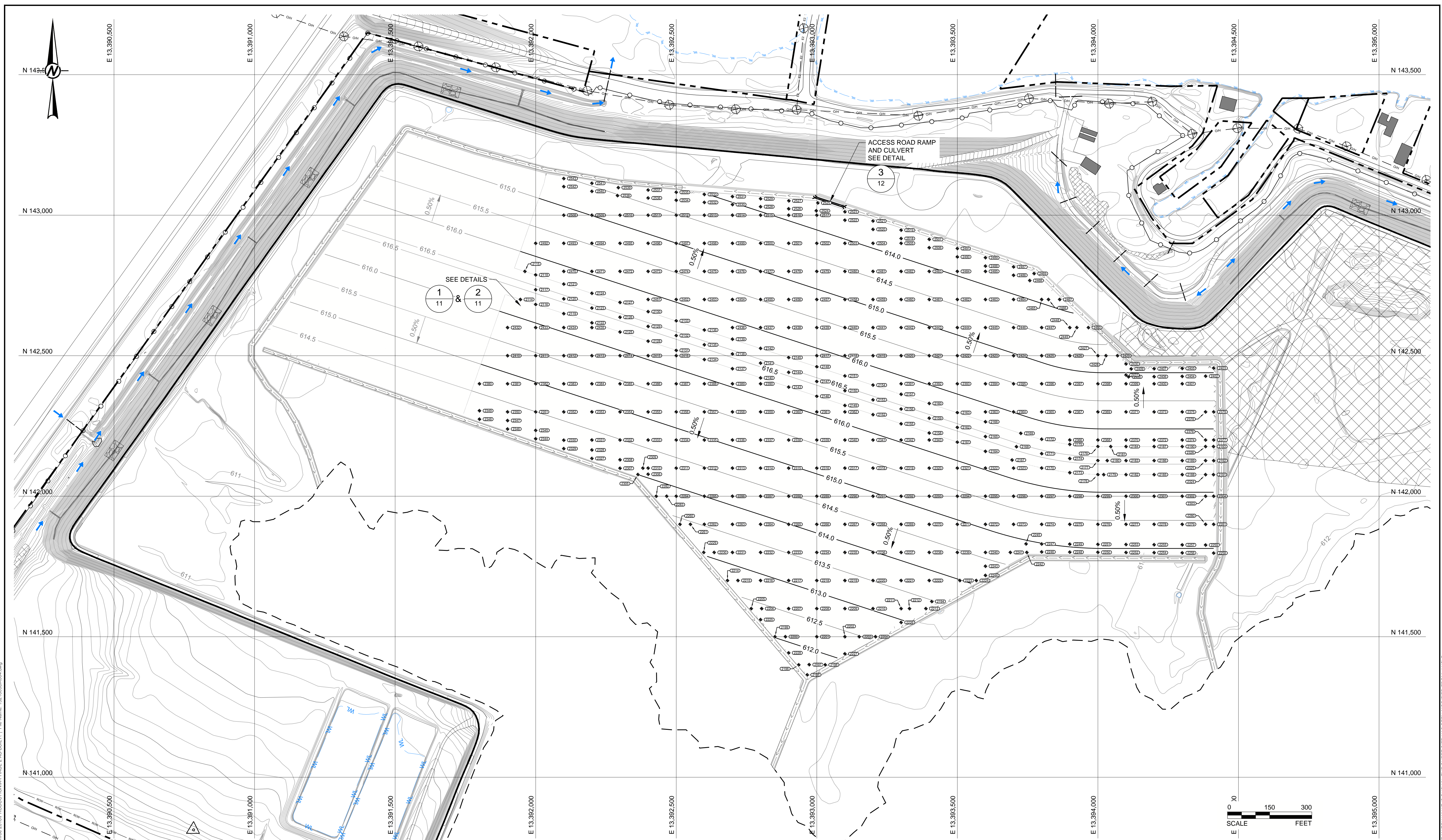
PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

TITLE
RECORD PORE WATER RELIEF PIPING PLAN

PROJECT No.	CONTROL	Rev.	FIGURE
1521809D	1521809DA004.dwg	0	5

Path: \\laning\cadd\Projects\1521809D_DTE_Monroe_2017\PRODUCTION\PHASE 2 AS-BUILT - File Name: 1521809DA004.dwg

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI D



LAYER PROFILE

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DTE ENERGY
MONROE POWER PLANT
MONROE, MI
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DESIGN	JJS
REVIEW	SEF
APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS
 TITLE
RECORD PORE WATER RELIEF SYSTEM PLAN

PROJECT No.	CONTROL	Rev.	FIGURE
1521809D	1521809DA004.dwg	0	6

Path: \\laning\cad\Projects\1521809D_DTE_Monroe_2016\PRODUCTION\PHASE 2 AS-BUILT_1 File Name: 1521809DA004.dwg

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANS/D

Path: \\waring\cadd\Projects\1521809D - DTE Monroe 2016\PRODUCTION\PHASE 2 AS-BUILT - File Name: 1521809DA004.dwg

Table with 6 columns: POINT NUMBER, NORTHING, EASTING, DESIGN ELEVATION, RECORD ELEVATION, DIFFERENCE. Contains data for points 2114 through 2208.

Table with 6 columns: POINT NUMBER, NORTHING, EASTING, DESIGN ELEVATION, RECORD ELEVATION, DIFFERENCE. Contains data for points 2209 through 2303.

Table with 6 columns: POINT NUMBER, NORTHING, EASTING, DESIGN ELEVATION, RECORD ELEVATION, DIFFERENCE. Contains data for points 2304 through 2386.

Table with 6 columns: POINT NUMBER, NORTHING, EASTING, DESIGN ELEVATION, RECORD ELEVATION, DIFFERENCE. Contains data for points 2387 through 2469.

Table with 6 columns: POINT NUMBER, NORTHING, EASTING, DESIGN ELEVATION, RECORD ELEVATION, DIFFERENCE. Contains data for points 2470 through 2543.

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MONROE POWER PLANT
MONROE, MI
CONSULTANT

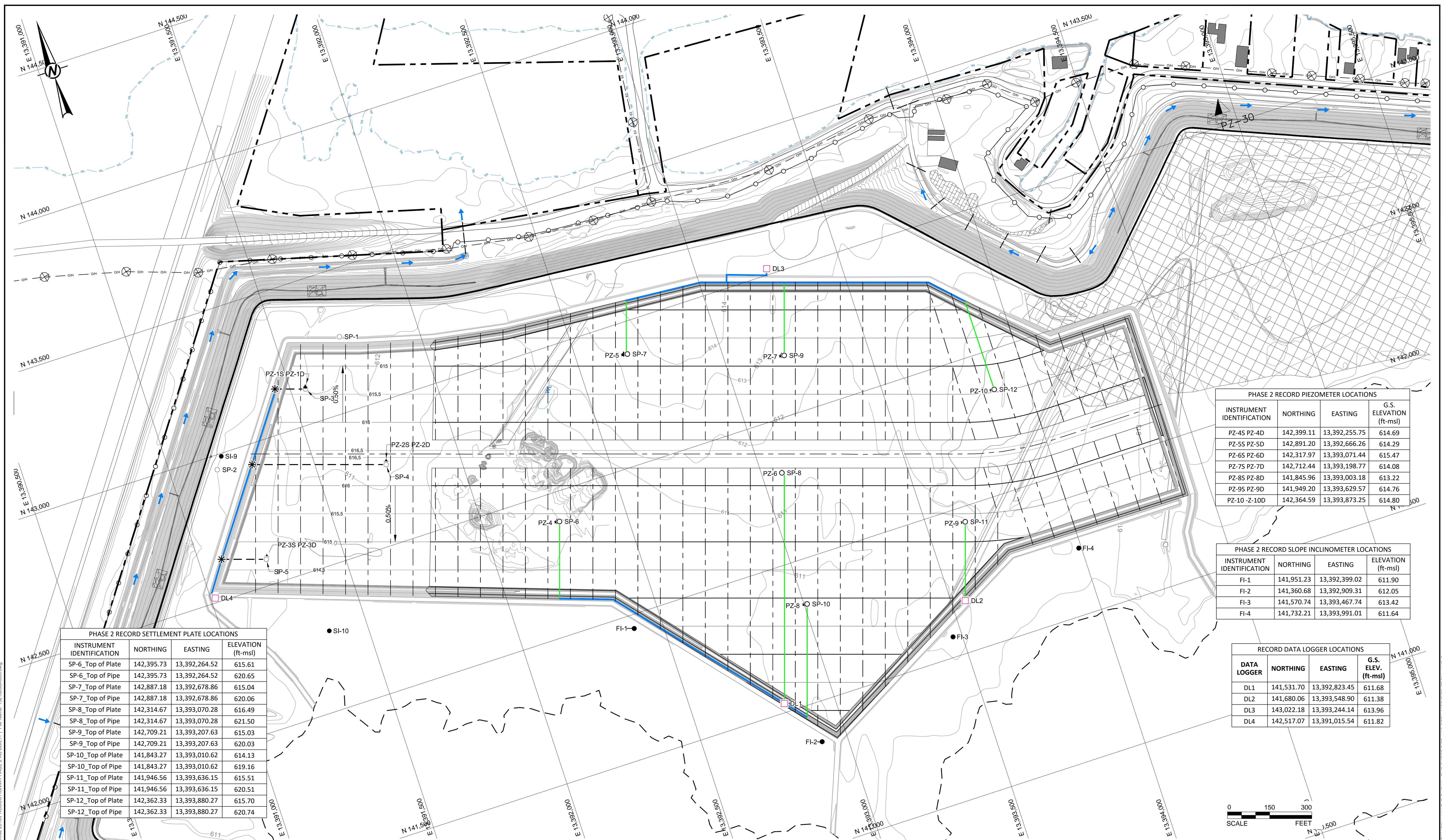


YYYY-MM-DD 2017-11-20
PREPARED DJC
DESIGN SF
REVIEW SEF
APPROVED DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS
TITLE
RECORD PORE WATER RELIEF SYSTEM
CONSTRUCTION CONTROL / QA/QC POINTS

PROJECT No. 1521809D CONTROL 1521809DA004.dwg Rev. 0 FIGURE 9

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSII



PHASE 2 RECORD PIEZOMETER LOCATIONS			
INSTRUMENT IDENTIFICATION	NORTHING	EASTING	G.S. ELEVATION (ft-msl)
PZ-4S PZ-4D	142,399.11	13,392,255.75	614.69
PZ-5S PZ-5D	142,891.20	13,392,666.26	614.29
PZ-6S PZ-6D	142,317.97	13,393,071.44	615.47
PZ-7S PZ-7D	142,712.44	13,393,198.77	614.08
PZ-8S PZ-8D	141,845.96	13,393,003.18	613.22
PZ-9S PZ-9D	141,949.20	13,393,629.57	614.76
PZ-10-Z-10D	142,364.59	13,393,873.25	614.80

PHASE 2 RECORD SLOPE INCLINOMETER LOCATIONS			
INSTRUMENT IDENTIFICATION	NORTHING	EASTING	ELEVATION (ft-msl)
FI-1	141,951.23	13,392,399.02	611.90
FI-2	141,360.68	13,392,909.31	612.05
FI-3	141,570.74	13,393,467.74	613.42
FI-4	141,732.21	13,393,991.01	611.64

PHASE 2 RECORD SETTLEMENT PLATE LOCATIONS			
INSTRUMENT IDENTIFICATION	NORTHING	EASTING	ELEVATION (ft-msl)
SP-6_Top of Plate	142,395.73	13,392,264.52	615.61
SP-6_Top of Pipe	142,395.73	13,392,264.52	620.65
SP-7_Top of Plate	142,887.18	13,392,678.86	615.04
SP-7_Top of Pipe	142,887.18	13,392,678.86	620.06
SP-8_Top of Plate	142,314.67	13,393,070.28	616.49
SP-8_Top of Pipe	142,314.67	13,393,070.28	621.50
SP-9_Top of Plate	142,709.21	13,393,207.63	615.03
SP-9_Top of Pipe	142,709.21	13,393,207.63	620.03
SP-10_Top of Plate	141,843.27	13,393,010.62	614.13
SP-10_Top of Pipe	141,843.27	13,393,010.62	619.16
SP-11_Top of Plate	141,946.56	13,393,636.15	615.51
SP-11_Top of Pipe	141,946.56	13,393,636.15	620.51
SP-12_Top of Plate	142,362.33	13,393,880.27	615.70
SP-12_Top of Pipe	142,362.33	13,393,880.27	620.74

RECORD DATA LOGGER LOCATIONS			
DATA LOGGER	NORTHING	EASTING	G.S. ELEV. (ft-msl)
DL1	141,531.70	13,392,823.45	611.68
DL2	141,680.06	13,393,548.90	611.38
DL3	143,022.18	13,393,244.14	613.96
DL4	142,517.07	13,391,015.54	611.82

- LEGEND**
- SI-1 PHASE 1 SLOPE INCLINOMETER
 - ▲ PZ-1D PHASE 1 VIBRATING WIRE PIEZOMETER (VWP) PAIR (DEEP & SHALLOW)
 - SP-1 PHASE 1 SETTLEMENT PLATE
 - * PHASE 1 REMOTE (CABLED) DATA LOGGER LOCATION FOR VWP
 - DATA LOGGER CABLE
 - RECORD LOCATION OF DRAINAGE PIPING
 - FI-1 2017 SLOPE INCLINOMETER
 - ★ 2017 VIBRATING WIRE PIEZOMETER (VWP) PAIR (DEEP & SHALLOW) AND SETTLEMENT PLATE
 - DATA TRANSMISSION LOCATION
 - CABLING OUTSIDE OVERLINER
 - CABLING INSIDE OVERLINER

CLIENT
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MONROE POWER PLANT
MONROE, MI

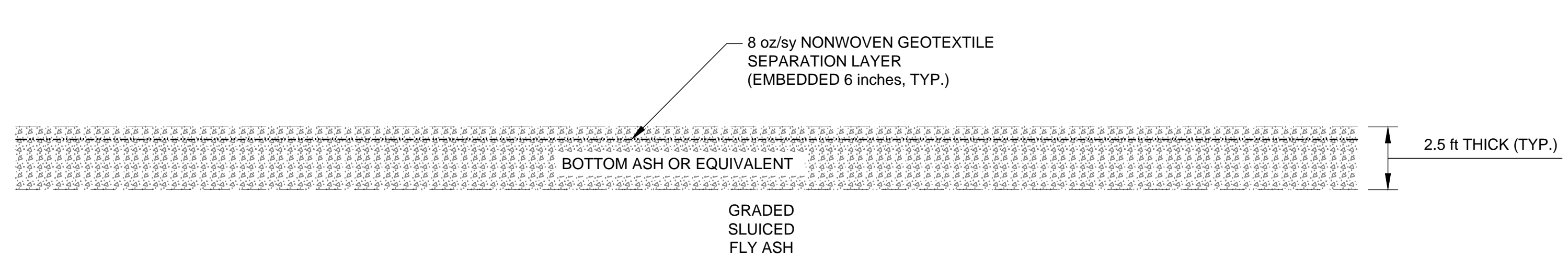
CONSULTANT	YYYY-MM-DD	2017-11-20
	PREPARED	JJS
	DESIGN	JJS
	REVIEW	SEF
	APPROVED	DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

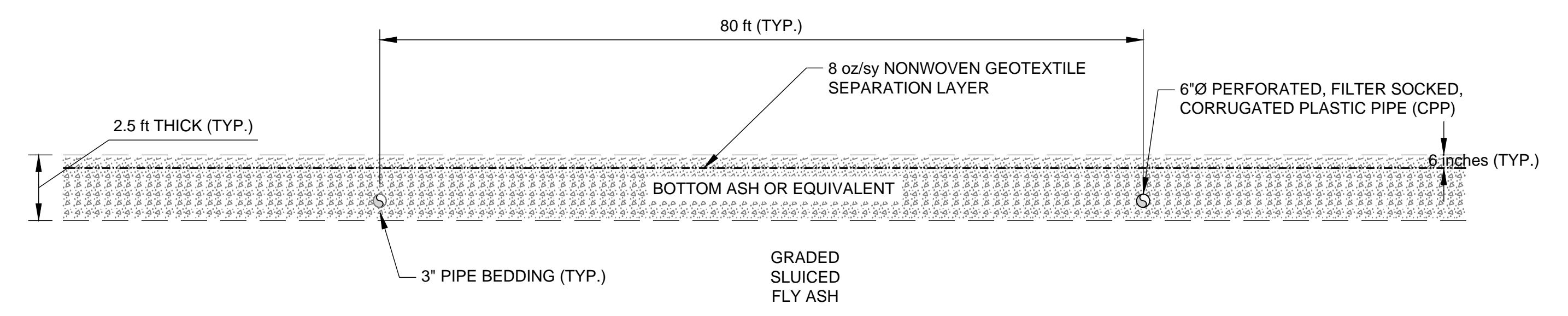
TITLE
RECORD MONITORING LOCATIONS FOR PHASE 2

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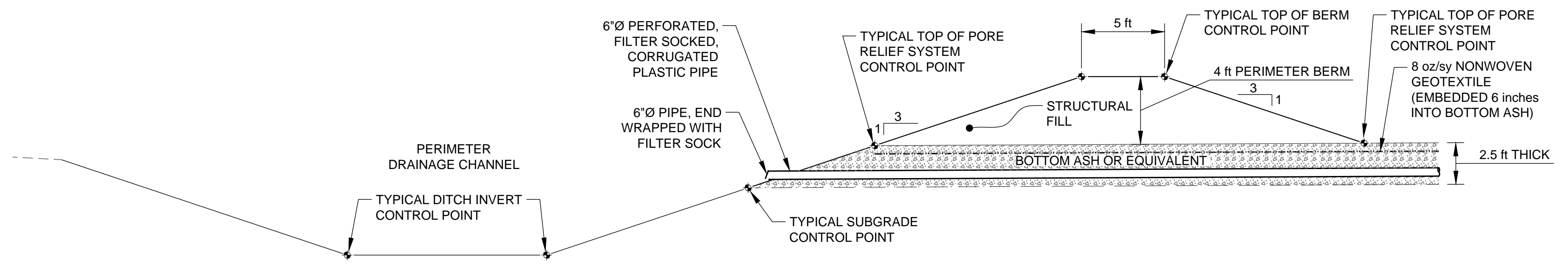
IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSIS 11x17



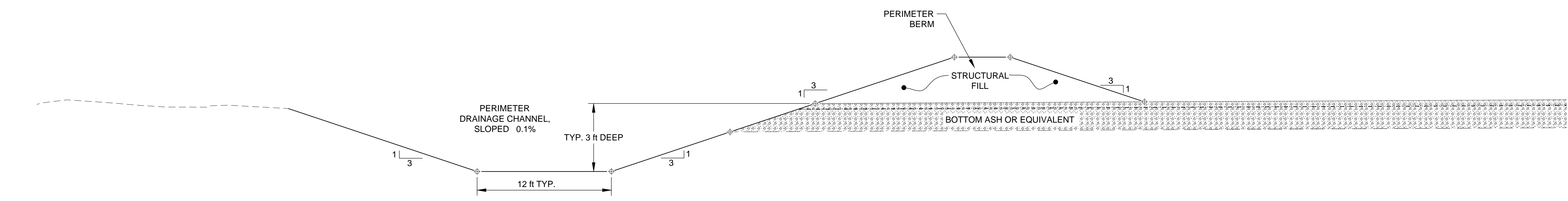
1 PORE RELIEF SYSTEM BASE LINER
11



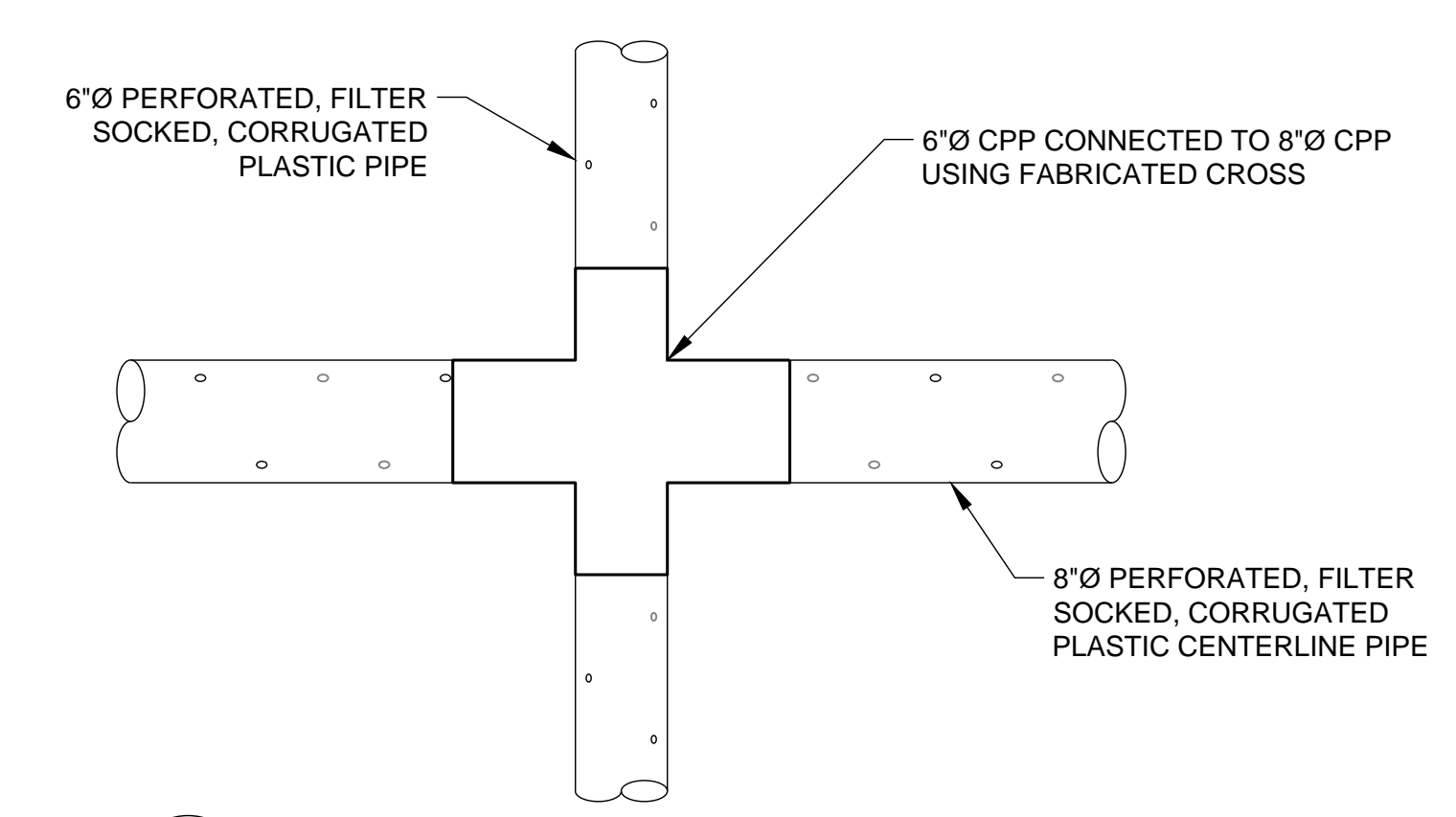
2 PORE RELIEF SYSTEM BASE LINER w/PIPES
11



3 PERIMETER BERM WITH PORE RELIEF PIPING SHOWN
11



4 PERIMETER BERM WITHOUT PIPING
11



5 CORRUGATED PLASTIC PIPE CENTERLINE CONNECTION
11

Path: \\anring\cad\Projects\1521809D_DTE_Monroe_2016\PRODUCTION\A-PHASE 2 AS-BUILT... File Name: 1521809DA006.dwg

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MONROE POWER PLANT
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CONSULTANT



YYYY-MM-DD 2017-11-20
PREPARED JJS
DESIGN JJS
REVIEW SEF
APPROVED DML

PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS

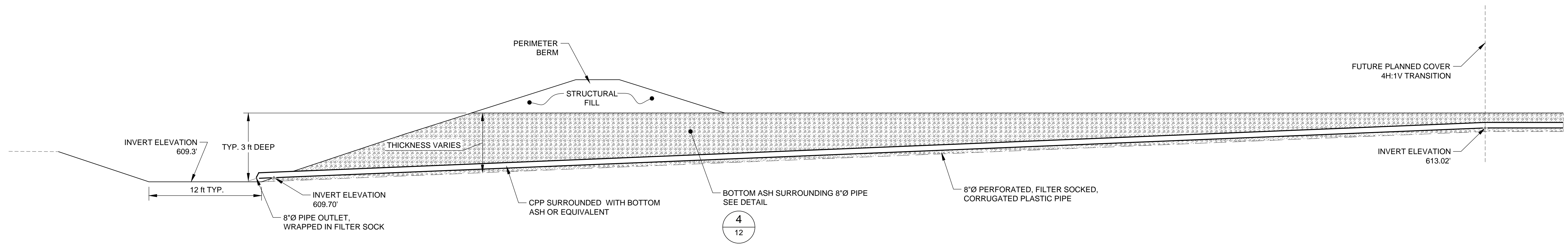
TITLE
GENERAL DETAILS
SHEET 1

PROJECT No. 1521809D CONTROL 1521809DA006.dwg

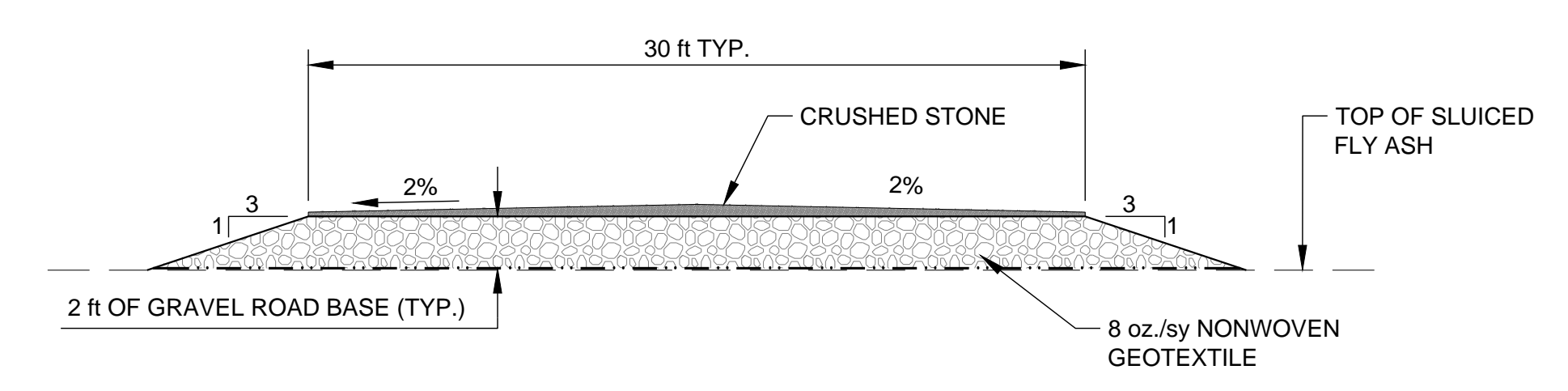
Rev. 0

FIGURE 11

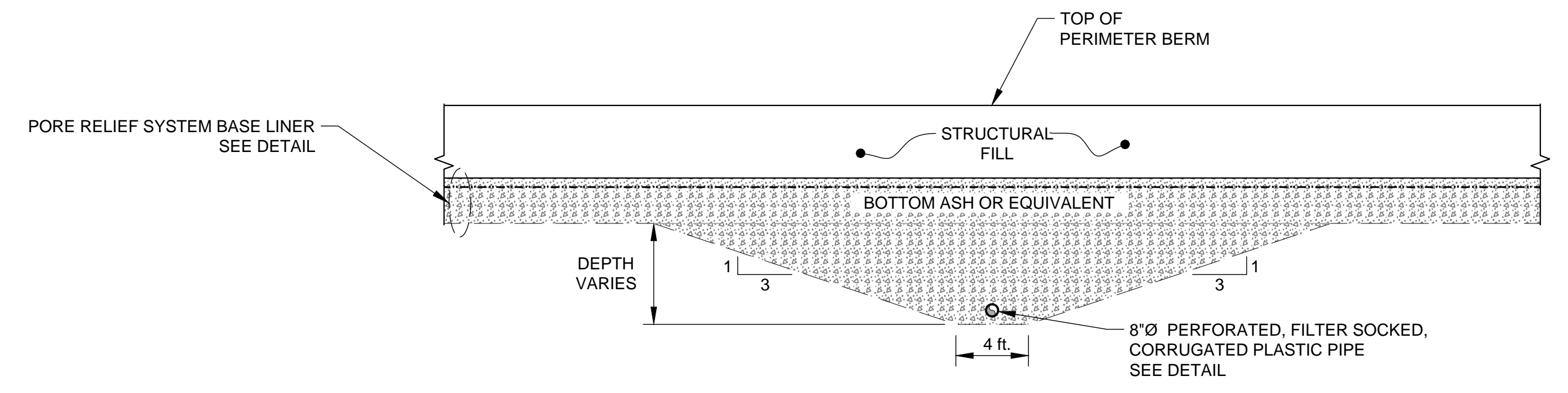
IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D



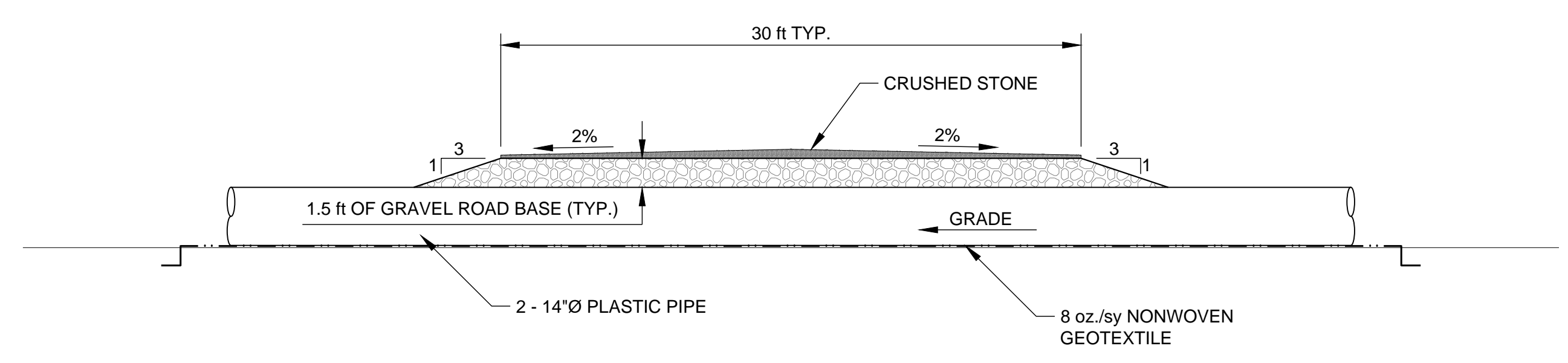
1 PERIMETER BERM WITH EASTERN CENTRAL PORE PRESSURE RELIEF PIPE SHOWN



2 TYPICAL ROAD SECTION



4 8"Ø PIPE TRENCH PAST 4H 1V SLOPE



3 ACCESS ROAD RAMP AND CULVERT DETAIL

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PROJECT
MONROE POWER PLANT ASH BASIN
2017 PHASE 2 RECORD DRAWINGS
TITLE
GENERAL DETAILS
SHEET 2

PROJECT No. 1521809D CONTROL 1521809DA006.dwg Rev. 0 FIGURE 12

Path: \\anring\cad\Projects\1521809D_DTE_Monroe_2016\PRODUCTION\A-PHASE 2 AS-BUILT... File Name: 1521809DA006.dwg

IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI D 11x17



**SECTION VI
ADDENDUMS AND SPECIFICATIONS CHANGES**

At Golder Associates we strive to be the most respected global group of companies specializing in ground engineering and environmental services. Employee owned since our formation in 1960, we have created a unique culture with pride in ownership, resulting in long-term organizational stability. Golder professionals take the time to build an understanding of client needs and of the specific environments in which they operate. We continue to expand our technical capabilities and have experienced steady growth with employees now operating from offices located throughout Africa, Asia, Australasia, Europe, North America and South America.

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Australasia	+ 61 3 8862 3500
Europe	+ 356 21 42 30 20
North America	+ 1 800 275 3281
South America	+ 55 21 3095 9500

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Golder Associates Inc.
15851 South US 27, Suite 50
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Tel: (517) 482-2262
Fax: (517) 482-2460



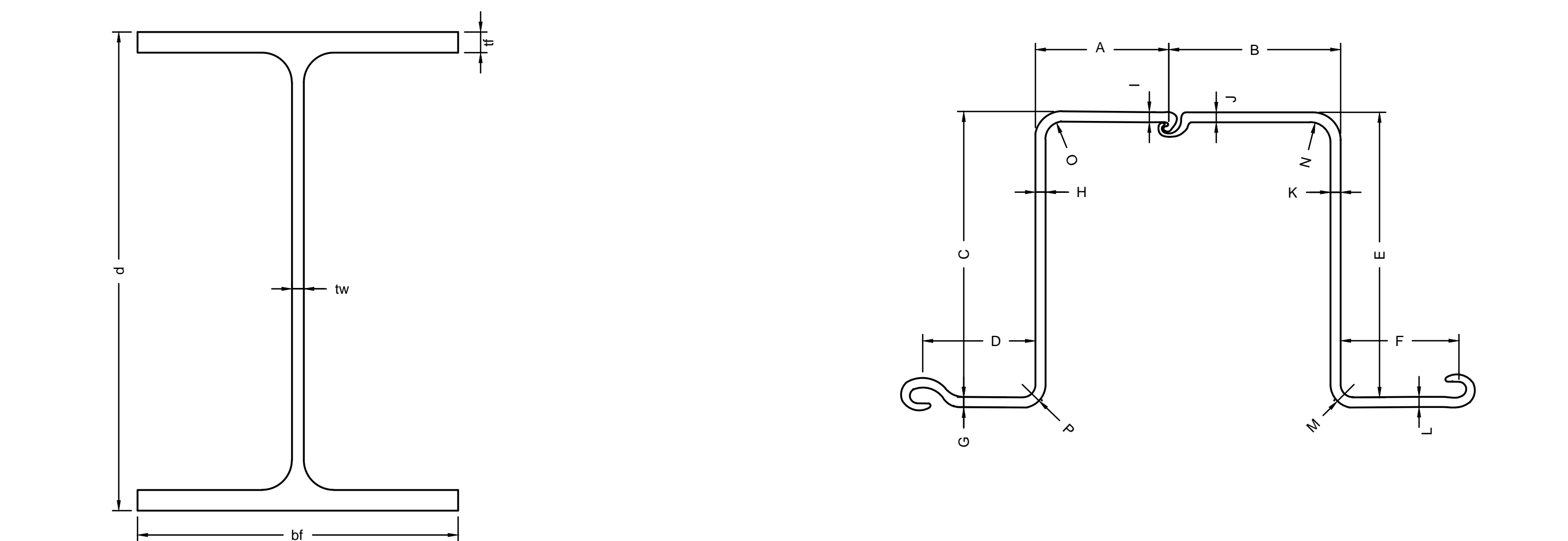
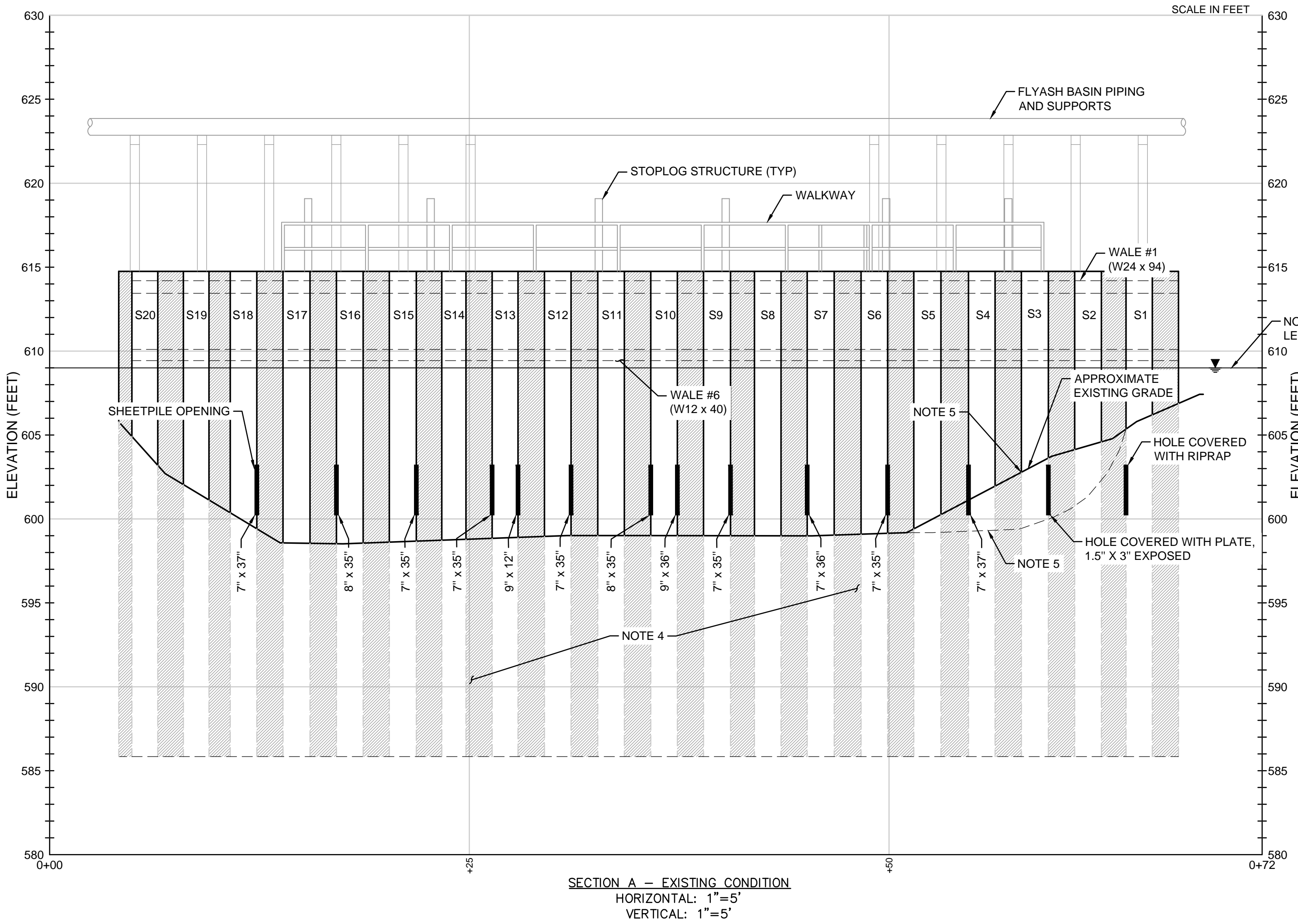
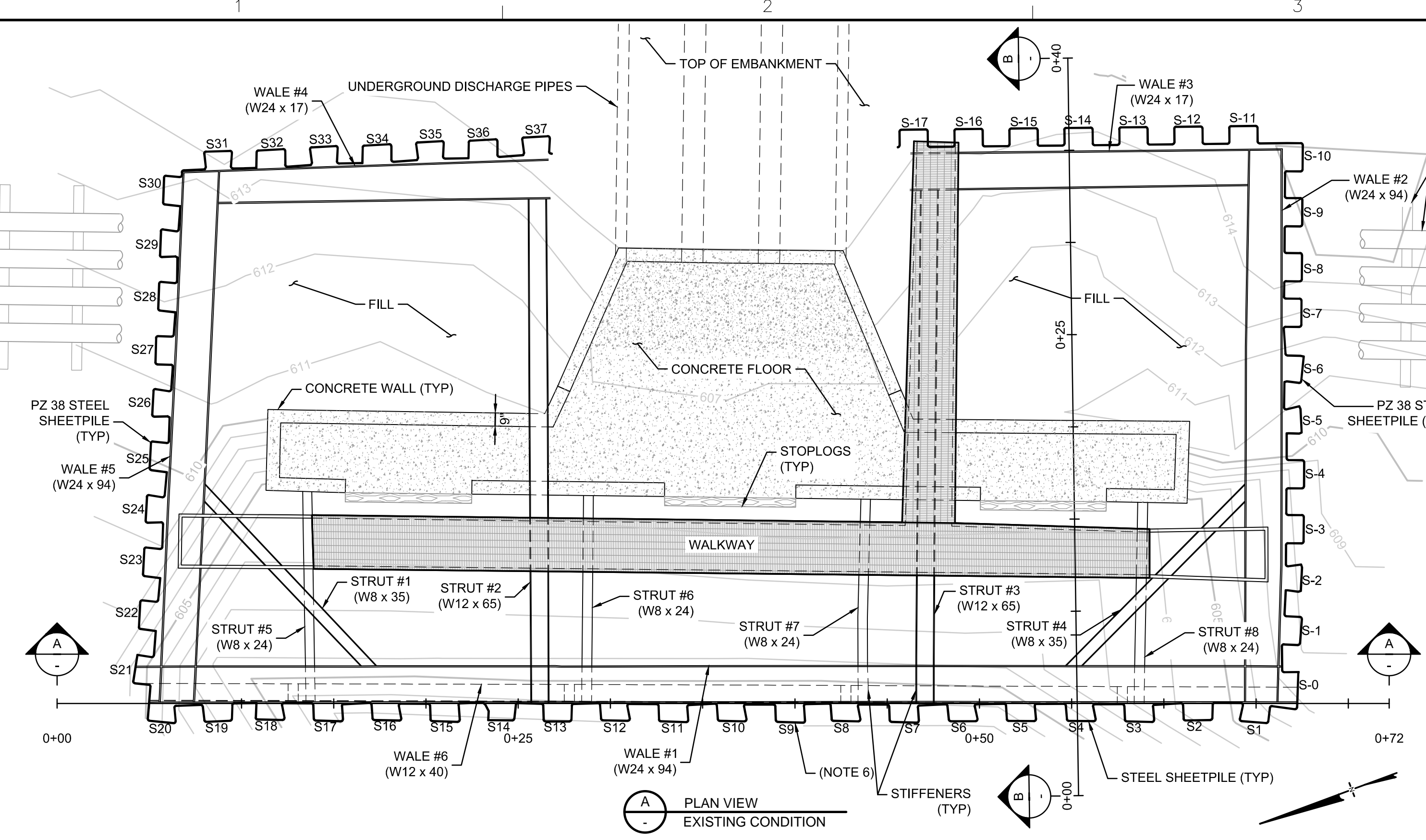


Table with columns: NUMBER, STRUCTURE DESIGNATION, bf, d, tf, tw. Lists structural members like WALE 1 (W24 x 94), STRUT 1 (W8 x 35), etc.

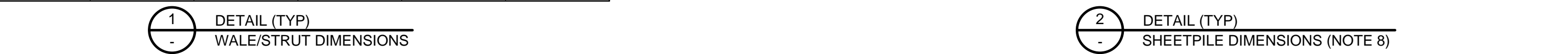


Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25.



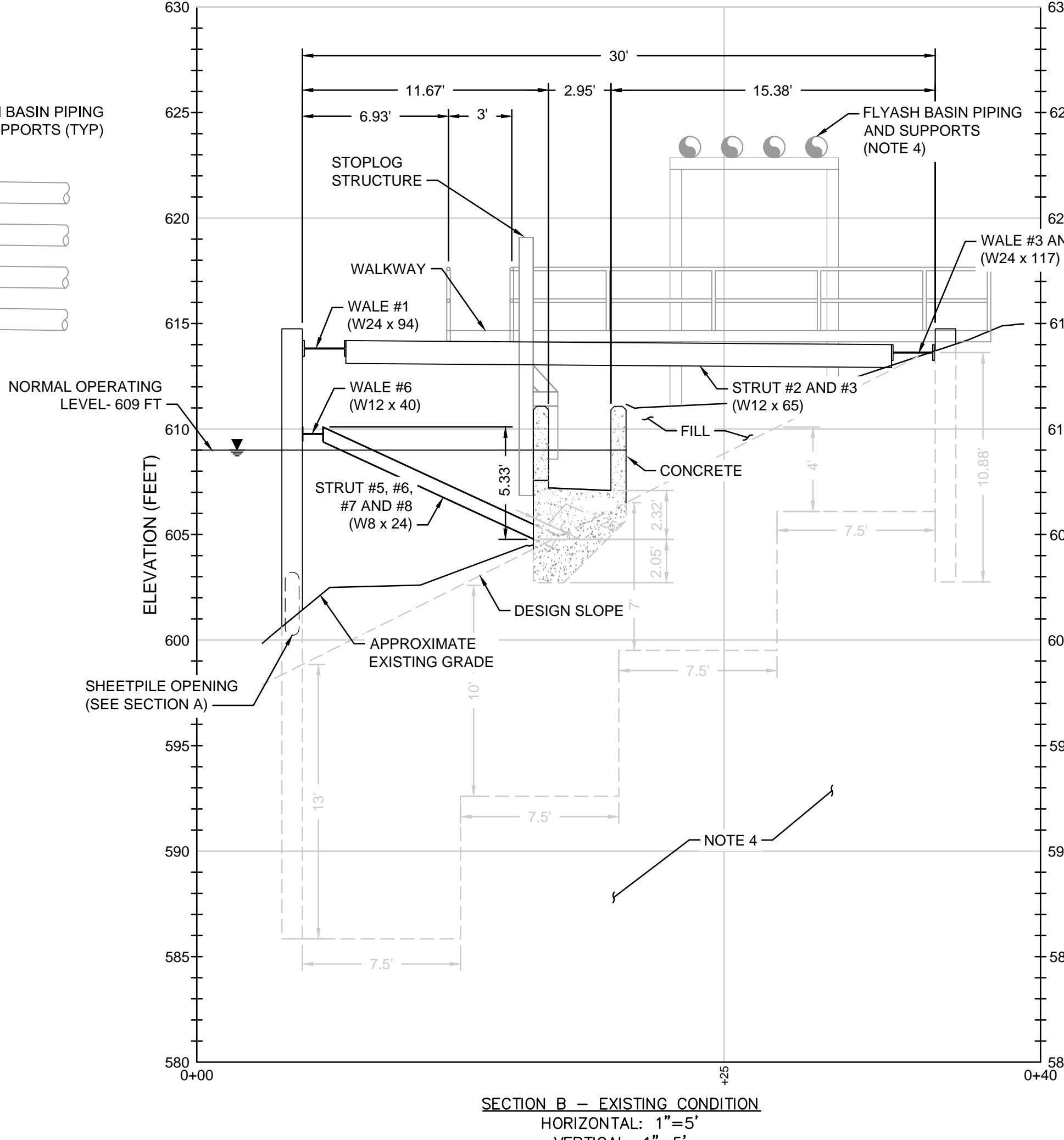
Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).



Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).



Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).



SECTION B - EXISTING CONDITION
HORIZONTAL: 1"=5'
VERTICAL: 1"=5'

- EXISTING CONDITION NOTES:
1. THIS DRAWING WAS PREPARED BASED ON MEASUREMENTS AND SURVEY OF THE DISCHARGE STRUCTURE...
2. SURVEY INFORMATION IS NOT PROVIDED ON THIS DRAWING...
3. THE ELEVATIONS ARE IN NATIONAL GEODETIC VERTICAL DATUM (NGVD) OF 1929.
4. FEATURES ABOVE GRADE AND THOSE ABOVE GRADE SHOWN IN GRAY ARE BASED ON THE INFORMATION PROVIDED IN THE HISTORICAL DRAWINGS LISTED IN NOTE 1.
5. APPROXIMATE EXISTING GRADE WAS ESTABLISHED BASED ON A FEW SURVEY POINTS...
6. WALE, STRUT AND SHEETPILE SIZES ARE SUMMARIZED IN DETAILS 1 AND 2.
7. STRUCTURE DESIGNATIONS ARE ASSIGNED BASED ON FIELD MEASUREMENTS...
8. ONLY CERTAIN SHEETPILE SECTIONS WERE SURVEYED...
9. SHEETPILES S0 AND S20 WERE FIELD-FIT TO CONSTRUCT THE CORNER...

DESIGN NOTES:

- DESIGN NOTES:
1. INSTALL 1" Ø A36 GALVANIZED THREADED RODS (RODS) AS SHOWN ON PLAN VIEW AND SECTION VIEW...
2. NEW HOLES TO EXISTING SHEETPILE TO BE FIELD DRILLED...
3. GRIND AND REMOVE EXISTING WELD (IF PRESENT) TO AVOID INTERFERENCE...
4. SOME OF THE RODS NEED TO BE INSTALLED AT AN ANGLE...
5. INSTALL THREE PAIRS OF RODS AT CORNERS AS SHOWN ON PLANVIEW B.

CONSTRUCTION SEQUENCE:

- CONSTRUCTION SEQUENCE:
A. DTE WILL MONITOR THE EXISTING DISCHARGE STRUCTURE BEFORE, DURING, AND AFTER THE CONSTRUCTION...
B. INSTALL THREADED RODS AS FOLLOWS:
- GRIND THE SURFACE OF THE SHEET PILE AND WALE AT THE LOCATION...
- INSTALL SNUG-TIGHTENED THREADED RODS...
- ESTABLISH A TIGHT CONNECTION BETWEEN SHEETPILE AND WALES...
- OBTAIN THE DTE SITE REPRESENTATIVE'S APPROVAL...
C. CUT SHEETPILES TO DESIGN CONDITION...
D. INSPECT UNDERWATER SHEETPILE OPENINGS...

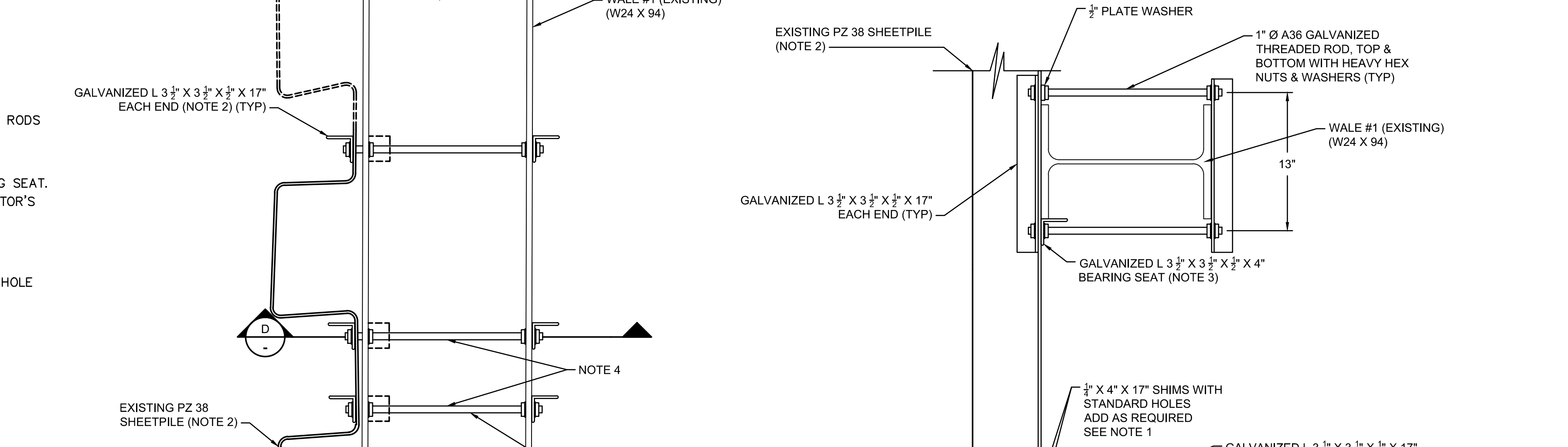
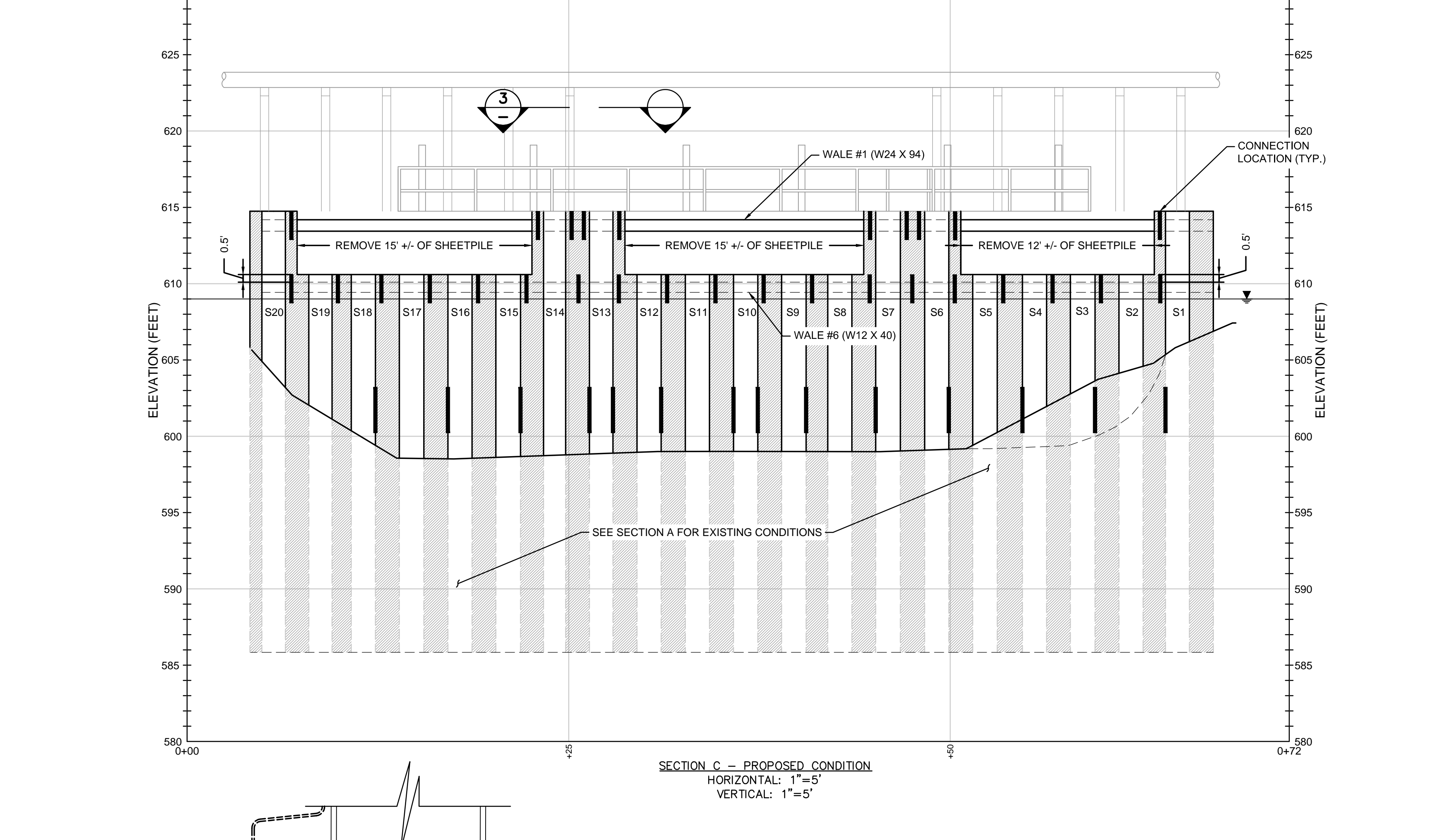
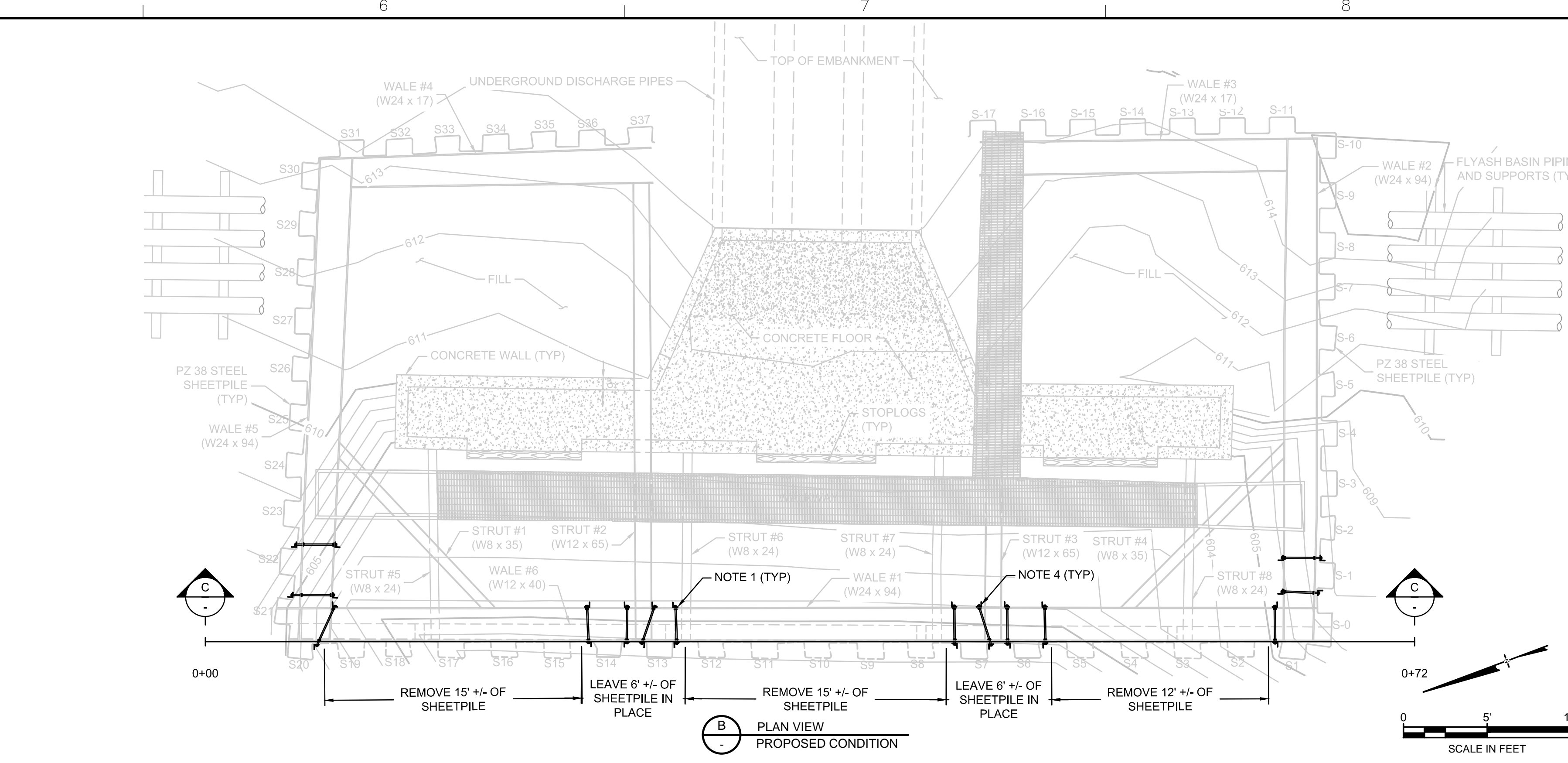


Table with columns: NUMBER, STRUCTURE DESIGNATION, bf, d, tf, tw. Lists structural members like WALE 1 (EXISTING), STRUT 3 (EXISTING), etc.



Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25.



Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).

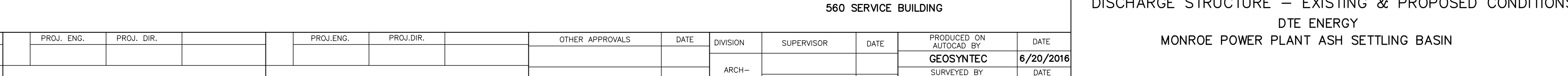


Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).

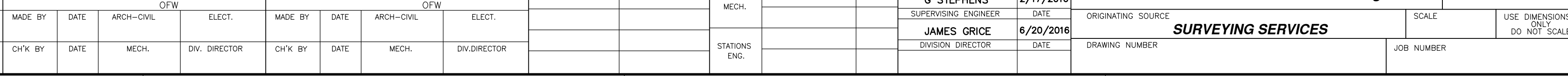
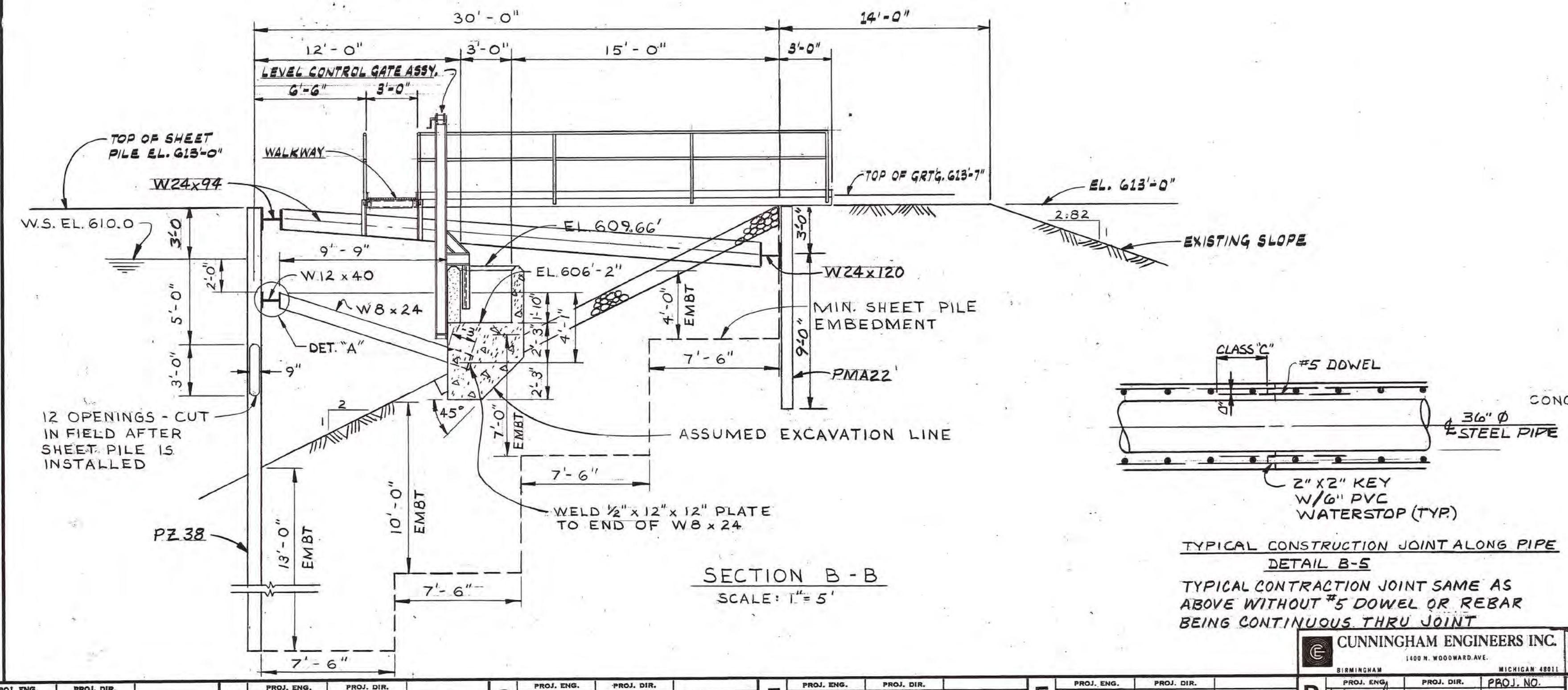
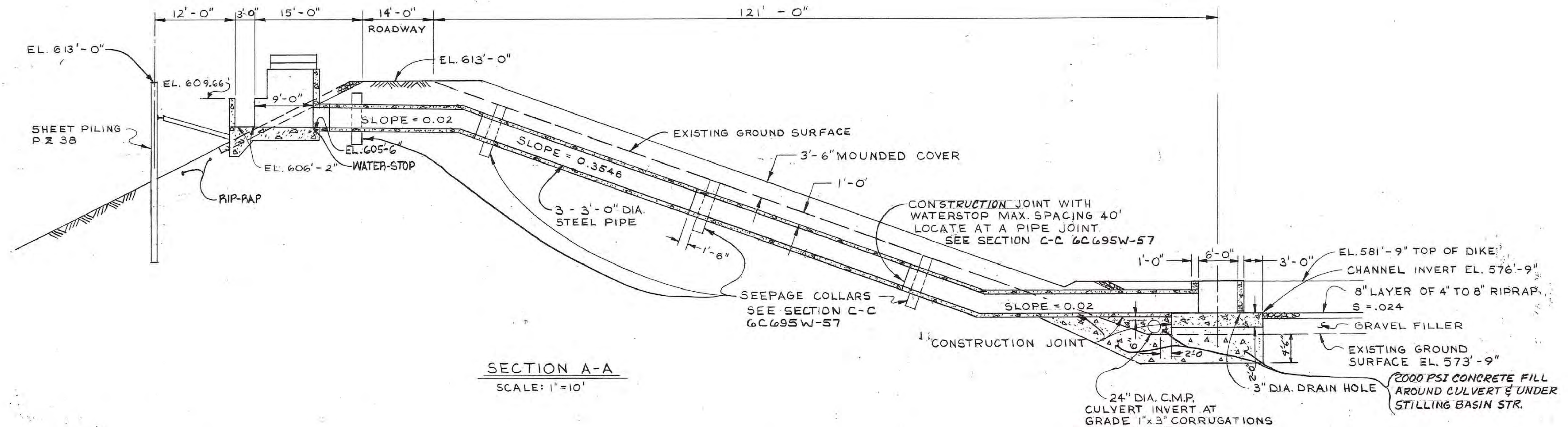
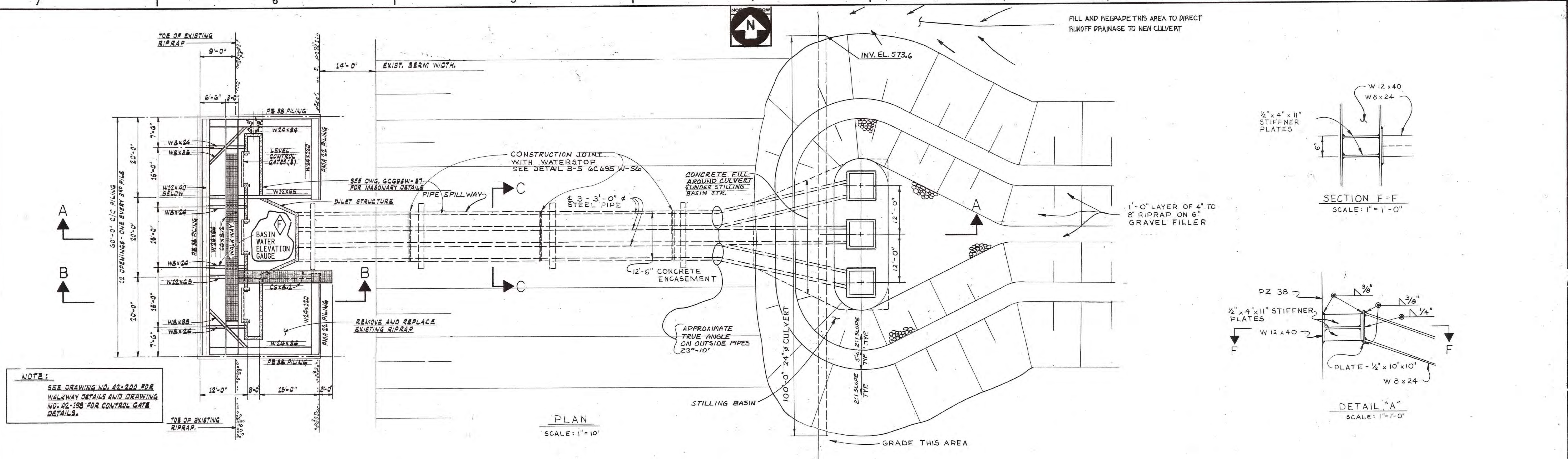


Table with columns: NUMBER, A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P. Lists sheetpile dimensions for members S-6 through S-25 (continued).

H:\PROJECTS\DISCHARGE STRUCTURE - CHERNIAK-14-10\DRAWINGS\PLAN SET\MDSD-14

THREE FULL WORKING DAYS BEFORE YOU CAN CALL THE PRESS DIG SYSTEM AT 1-800-443-7371

Approval stamps and project information including: DETROIT EDISON - A DTE ENERGY COMPANY, DISCHARGE STRUCTURE - EXISTING & PROPOSED CONDITIONS, DETROIT EDISON'S SURVEYING SERVICES, 560 SERVICE BUILDING, PROJECT: MONROE POWER PLANT ASH SETTLING BASIN, DTE ENERGY, SURVEYING ENGINEER: G STEPHENS, DATE: 2/17/2016, SURVEYING ENGINEER: JAMES ORICE, DATE: 6/20/2016.



6C695W-56
LATEST REVISION

PROJ. ENGR.	PROJ. DIR.	DATE	REVISION	DESCRIPTION
I				
H				
G				
F				ADDED LOCATION OF WATER LEVEL GAUGE
E				ADDED BASIN LEVEL CONTROL GATES & PLATFORM
D				REVISED DRAWING TO DEPICT AS BUILT CONDITIONS
C				ADDED UPPER WHALER AND ANCHORAGE. LOWERED TOP OF SHEET PILING 3'-0" TO EL. 613'-0"
B				ADDED SEEPAGE COLLAR, CHANGED PIPE DEPTH, CHANGED ROADWAY WIDTH, 3000 CONCRETE FILL UNDER STILLING BASIN STR.
A				ISSUE INLET STR. PIPE SPILLWAY AND STILLING BASIN FOR CONST. 6/24/77

DATE	6/21/77	BY	C.T. BERGMAN
DATE	6/21/77	BY	M.L.K.
DATE	6/21/77	BY	J.H.M.
DATE	6/21/77	BY	L.D.C.
DATE	6/21/77	BY	G.K.E.

PROJ. ENGR.	PROJ. DIR.	DATE	REVISION	DESCRIPTION
I				
H				
G				
F				
E				
D				
C				
B				
A				

DATE	6/21/77	BY	C.T. BERGMAN
DATE	6/21/77	BY	M.L.K.
DATE	6/21/77	BY	J.H.M.
DATE	6/21/77	BY	L.D.C.
DATE	6/21/77	BY	G.K.E.

DATE	6/21/77	BY	C.T. BERGMAN
DATE	6/21/77	BY	M.L.K.
DATE	6/21/77	BY	J.H.M.
DATE	6/21/77	BY	L.D.C.
DATE	6/21/77	BY	G.K.E.

DATE	6/21/77	BY	C.T. BERGMAN
DATE	6/21/77	BY	M.L.K.
DATE	6/21/77	BY	J.H.M.
DATE	6/21/77	BY	L.D.C.
DATE	6/21/77	BY	G.K.E.

MODIFICATIONS TO THE MONROE ASH BASIN EMBANKMENT CREST

9/23/2016

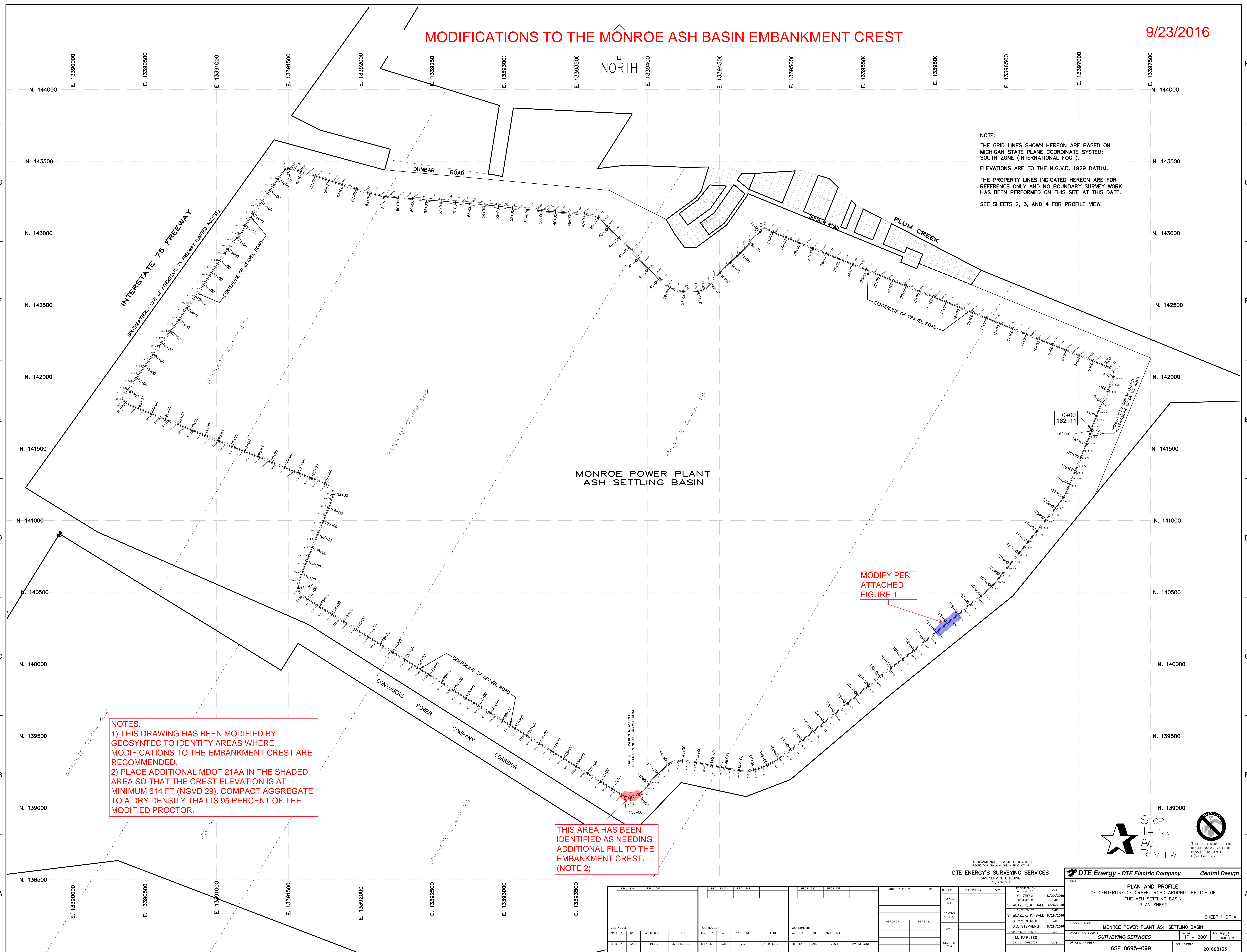


NOTE:
 THE GRID LINES SHOWN HEREON ARE BASED ON MICHIGAN STATE PLANE COORDINATE SYSTEM; SOUTH ZONE (INTERNATIONAL FOOT).
 ELEVATIONS ARE TO THE N.G.V.D. 1929 DATUM.
 THE PROPERTY LINES INDICATED HEREON ARE FOR REFERENCE ONLY AND NO BOUNDARY SURVEY WORK HAS BEEN PERFORMED ON THIS SITE AT THIS DATE.
 SEE SHEETS 2, 3, AND 4 FOR PROFILE VIEW.

NOTES:
 1) THIS DRAWING HAS BEEN MODIFIED BY GEOSYNTEC TO IDENTIFY AREAS WHERE MODIFICATIONS TO THE EMBANKMENT CREST ARE RECOMMENDED.
 2) PLACE ADDITIONAL MDOT 21AA IN THE SHADED AREA SO THAT THE CREST ELEVATION IS AT MINIMUM 614 FT (NGVD 29). COMPACT AGGREGATE TO A DRY DENSITY THAT IS 95 PERCENT OF THE MODIFIED PROCTOR.

THIS AREA HAS BEEN IDENTIFIED AS NEEDING ADDITIONAL FILL TO THE EMBANKMENT CREST. (NOTE 2)

MODIFY PER ATTACHED FIGURE 1

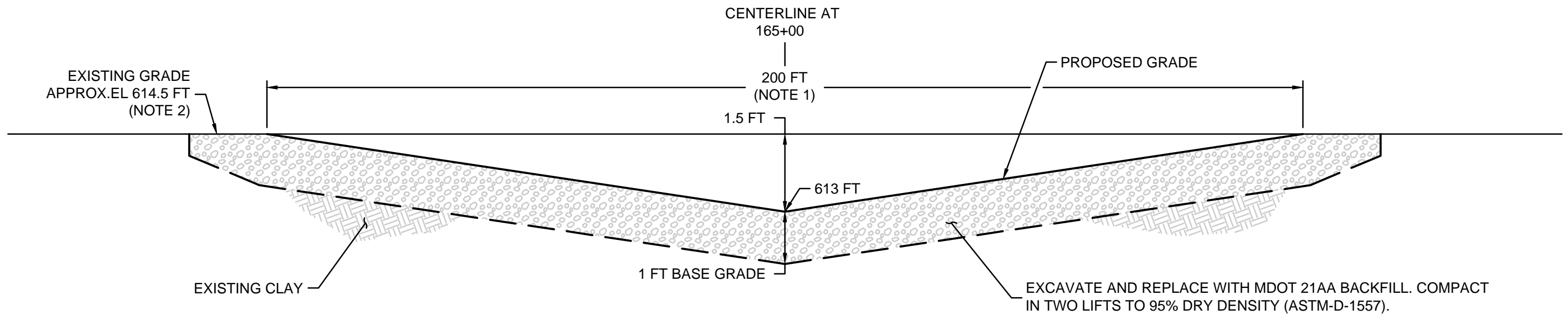


THIS DRAWING AND THE WORK PERFORMED TO CREATE THIS DRAWING ARE A PROPERTY OF
DTE ENERGY'S SURVEYING SERVICES
 545 SERVICE BUILDING
 (313) 435-3046

DTE Energy - DTE Electric Company Central Design	
PLAN AND PROFILE OF CENTERLINE OF GRAVEL ROAD AROUND THE TOP OF THE ASH SETTLING BASIN --PLAN SHEET--	
SHEET 1 OF 4	
LOCATION NAME: MONROE POWER PLANT ASH SETTLING BASIN	
SURVEYING SERVICES	
SCALE: 1" = 200' USE DIMENSIONS DO NOT SCALE	
DRAWING NUMBER: 6SE 0695-099	JOB NUMBER: 201608133

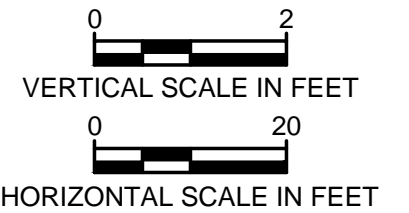
PROJ. ENG.	PROJ. DIR.	PROJ. ENG.	PROJ. DIR.	PROJ. ENG.	PROJ. DIR.	OTHER APPROVALS	DATE	DIVISION	SUPERVISOR	DATE	
JOB NUMBER: _____ MADE BY: _____ DATE: _____ ARCH-CIVIL _____ ELECT. _____ CHK BY: _____ DATE: _____ MECH. _____ DIV. DIRECTOR _____		JOB NUMBER: _____ MADE BY: _____ DATE: _____ ARCH-CIVIL _____ ELECT. _____ CHK BY: _____ DATE: _____ MECH. _____ DIV. DIRECTOR _____		JOB NUMBER: _____ MADE BY: _____ DATE: _____ ARCH-CIVIL _____ ELECT. _____ CHK BY: _____ DATE: _____ MECH. _____ DIV. DIRECTOR _____		REFERENCES: _____ REVISIONS: _____		DIVISION: _____ SUPERVISOR: _____ DATE: _____		PROJECTOR OR AUTOCAD BY: _____ DATE: _____ CHECKED BY: _____ DATE: _____ SURVEY ENGINEER: _____ DATE: _____ SUPERVISING ENGINEER: _____ DATE: _____ DIVISION DIRECTOR: _____ DATE: _____	

\\CHICAGO-01\PRJ1\CWP\CHE8242-DETROIT EDISON\500 - TECHNICAL\502 ASH POND STABILITY\502-13 STRUCTURAL STABILITY & HYDRAULICS ASSESSMENT\REPORTS REQUESTED BY DTE\HAZARD POTENTIAL LETTER\CREST MODIFICATION INSTRUCTIONS\DTE V-NOTCH



NOTES:

- 1) PITCH CREST 2% TOWARD THE ASH BASIN.
- 2) VERTICAL DATUM IS IN NGVD 29.



PROPOSED CREST MODIFICATION AT STATION 165+00	
PROJECT NO: CHE8242U	SEPTEMBER 2016
FIGURE 1	

APPENDIX A: STORM WATER CALCULATIONS

Appendix B.1: NOAA Rainfall Data



NOAA Atlas 14, Volume 8, Version 2
Location name: Monroe Twp, Michigan, USA*
Latitude: 41.8838°, Longitude: -83.376°
Elevation: 611.75 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.312 (0.247-0.395)	0.370 (0.293-0.468)	0.465 (0.367-0.589)	0.543 (0.427-0.689)	0.651 (0.497-0.840)	0.734 (0.551-0.954)	0.817 (0.597-1.08)	0.901 (0.637-1.21)	1.01 (0.694-1.37)	1.10 (0.737-1.50)
10-min	0.457 (0.362-0.578)	0.542 (0.429-0.685)	0.680 (0.537-0.862)	0.795 (0.625-1.01)	0.953 (0.728-1.23)	1.07 (0.806-1.40)	1.20 (0.874-1.58)	1.32 (0.933-1.76)	1.48 (1.02-2.01)	1.60 (1.08-2.20)
15-min	0.557 (0.441-0.705)	0.661 (0.523-0.836)	0.829 (0.655-1.05)	0.969 (0.762-1.23)	1.16 (0.888-1.50)	1.31 (0.983-1.70)	1.46 (1.07-1.92)	1.61 (1.14-2.15)	1.81 (1.24-2.45)	1.96 (1.32-2.68)
30-min	0.765 (0.606-0.967)	0.910 (0.720-1.15)	1.15 (0.904-1.45)	1.34 (1.05-1.70)	1.61 (1.23-2.08)	1.81 (1.36-2.36)	2.02 (1.47-2.66)	2.22 (1.57-2.97)	2.50 (1.71-3.39)	2.70 (1.82-3.71)
60-min	0.978 (0.775-1.24)	1.15 (0.912-1.46)	1.45 (1.14-1.83)	1.70 (1.33-2.15)	2.05 (1.57-2.66)	2.33 (1.76-3.05)	2.62 (1.92-3.47)	2.93 (2.08-3.93)	3.34 (2.30-4.55)	3.66 (2.46-5.02)
2-hr	1.19 (0.956-1.49)	1.40 (1.12-1.74)	1.74 (1.39-2.18)	2.05 (1.63-2.57)	2.49 (1.94-3.21)	2.85 (2.18-3.69)	3.23 (2.40-4.23)	3.63 (2.61-4.82)	4.18 (2.91-5.64)	4.62 (3.14-6.26)
3-hr	1.32 (1.07-1.64)	1.53 (1.24-1.90)	1.91 (1.54-2.37)	2.24 (1.80-2.79)	2.74 (2.16-3.52)	3.16 (2.43-4.07)	3.61 (2.70-4.70)	4.08 (2.96-5.41)	4.76 (3.34-6.39)	5.30 (3.62-7.14)
6-hr	1.56 (1.27-1.90)	1.78 (1.46-2.18)	2.19 (1.79-2.68)	2.57 (2.09-3.16)	3.16 (2.53-4.01)	3.66 (2.86-4.66)	4.20 (3.18-5.42)	4.79 (3.51-6.27)	5.63 (3.99-7.49)	6.32 (4.36-8.41)
12-hr	1.80 (1.49-2.17)	2.05 (1.70-2.47)	2.50 (2.06-3.01)	2.91 (2.40-3.53)	3.56 (2.88-4.46)	4.10 (3.24-5.16)	4.69 (3.60-5.98)	5.33 (3.95-6.90)	6.26 (4.48-8.22)	7.01 (4.88-9.21)
24-hr	2.06 (1.74-2.45)	2.35 (1.97-2.79)	2.85 (2.39-3.40)	3.31 (2.75-3.95)	3.99 (3.26-4.91)	4.56 (3.64-5.64)	5.16 (4.00-6.48)	5.82 (4.35-7.42)	6.75 (4.87-8.73)	7.49 (5.27-9.73)
2-day	2.38 (2.03-2.79)	2.70 (2.30-3.17)	3.26 (2.77-3.83)	3.75 (3.17-4.42)	4.47 (3.69-5.40)	5.05 (4.08-6.15)	5.67 (4.43-7.00)	6.32 (4.76-7.92)	7.22 (5.26-9.20)	7.93 (5.63-10.2)
3-day	2.61 (2.24-3.03)	2.94 (2.53-3.42)	3.52 (3.01-4.10)	4.02 (3.42-4.70)	4.75 (3.94-5.68)	5.34 (4.33-6.43)	5.95 (4.68-7.28)	6.60 (5.01-8.21)	7.49 (5.49-9.48)	8.20 (5.86-10.4)
4-day	2.80 (2.42-3.24)	3.15 (2.72-3.64)	3.73 (3.21-4.32)	4.24 (3.63-4.93)	4.98 (4.16-5.93)	5.58 (4.55-6.68)	6.19 (4.90-7.53)	6.84 (5.22-8.46)	7.74 (5.70-9.74)	8.45 (6.06-10.7)
7-day	3.31 (2.89-3.78)	3.68 (3.21-4.21)	4.32 (3.76-4.94)	4.87 (4.21-5.58)	5.65 (4.75-6.63)	6.27 (5.16-7.41)	6.91 (5.52-8.30)	7.58 (5.83-9.26)	8.50 (6.30-10.6)	9.21 (6.66-11.5)
10-day	3.76 (3.31-4.26)	4.17 (3.66-4.73)	4.85 (4.25-5.51)	5.43 (4.73-6.18)	6.25 (5.29-7.27)	6.90 (5.72-8.09)	7.57 (6.08-9.02)	8.26 (6.38-10.0)	9.20 (6.85-11.4)	9.93 (7.21-12.4)
20-day	5.12 (4.57-5.71)	5.60 (5.00-6.26)	6.41 (5.70-7.17)	7.08 (6.26-7.95)	8.01 (6.87-9.17)	8.74 (7.33-10.1)	9.47 (7.70-11.1)	10.2 (7.98-12.2)	11.2 (8.44-13.6)	12.0 (8.79-14.7)
30-day	6.30 (5.67-6.98)	6.89 (6.19-7.63)	7.83 (7.02-8.69)	8.60 (7.67-9.57)	9.65 (8.32-10.9)	10.4 (8.82-11.9)	11.2 (9.17-13.0)	12.0 (9.43-14.2)	13.0 (9.85-15.7)	13.8 (10.2-16.8)
45-day	7.87 (7.14-8.64)	8.61 (7.81-9.45)	9.77 (8.83-10.8)	10.7 (9.62-11.8)	11.9 (10.3-13.3)	12.8 (10.9-14.5)	13.6 (11.2-15.7)	14.5 (11.4-16.9)	15.5 (11.7-18.4)	16.2 (12.0-19.6)
60-day	9.24 (8.44-10.1)	10.1 (9.26-11.1)	11.5 (10.5-12.6)	12.6 (11.4-13.8)	14.0 (12.2-15.5)	15.0 (12.8-16.8)	15.8 (13.1-18.0)	16.7 (13.2-19.3)	17.6 (13.4-20.9)	18.3 (13.6-22.0)

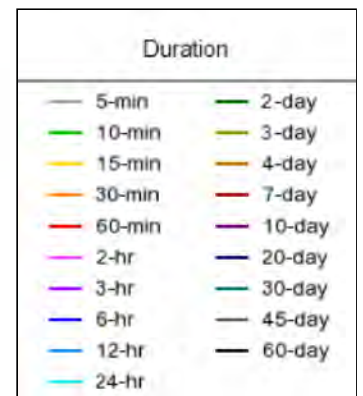
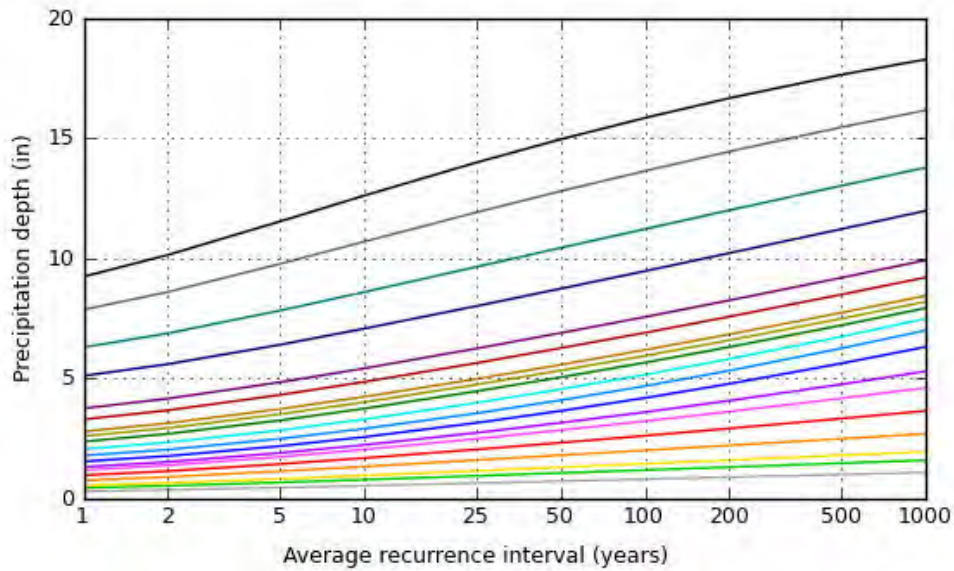
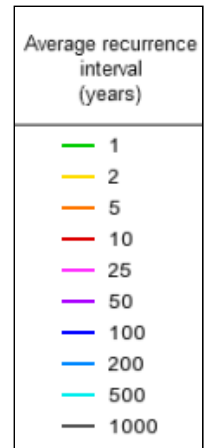
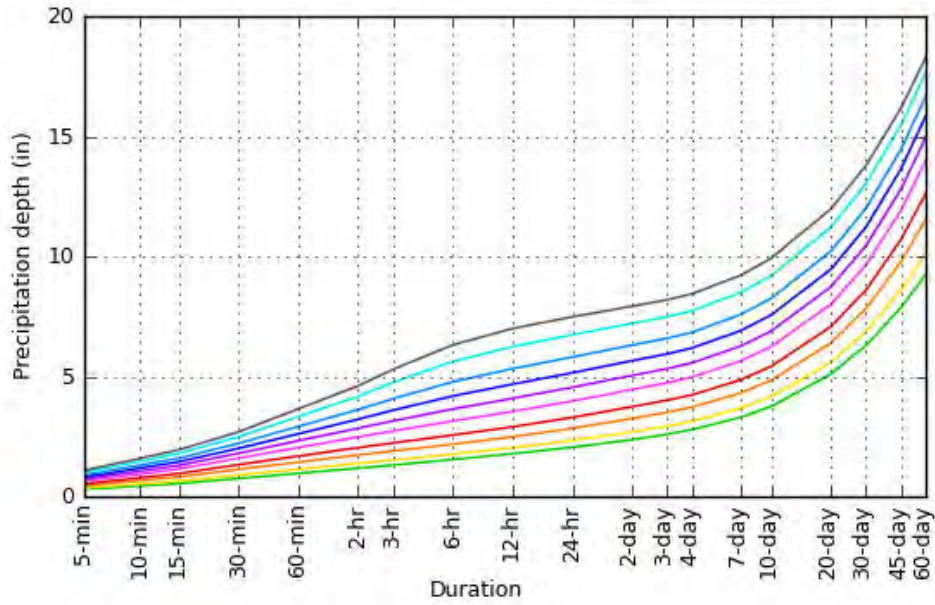
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

Latitude: 41.8838°, Longitude: -83.3760°



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Maps & aerials

Small scale terrain



Large scale terrain



Large scale map



Large scale aerial



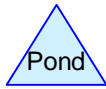
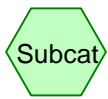
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1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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APPENDIX A: STORM WATER CALCULATIONS

Appendix B2: HydroCAD 25-year/24-hour Output



Routing Diagram for Monroe Fly Ash Basin_RORO_Draft 8.31.21

Prepared by AECOM, Printed 9/1/2021

HydroCAD® 10.00-20 s/n 01723 © 2017 HydroCAD Software Solutions LLC

Summary for Subcatchment 1S: NW Drainage area Upstream culvert

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 48.31 cfs @ 12.15 hrs, Volume= 3.579 af, Depth> 2.18"

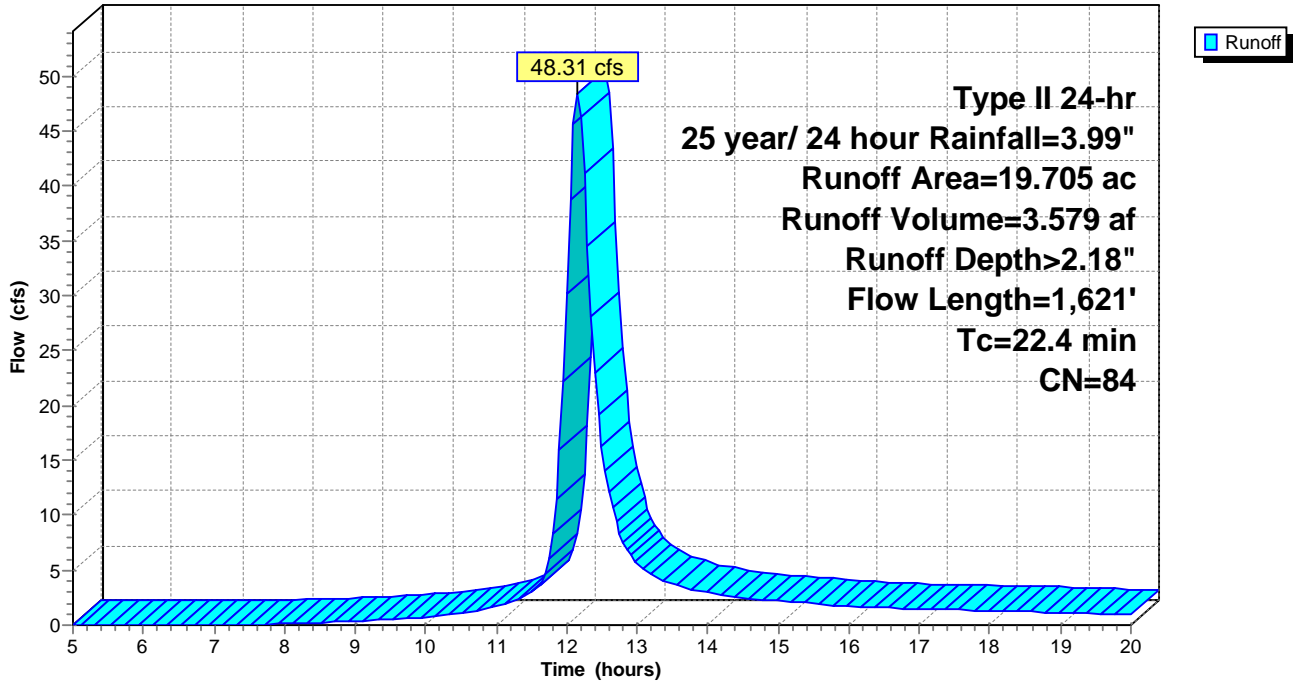
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 19.705	84	Clay cap with 6 inches of vegetative cover
19.705		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	100	0.0500	0.21		Sheet Flow, sheet flow 5% Grass: Short n= 0.150 P2= 2.35"
4.5	418	0.0500	1.57		Shallow Concentrated Flow, shallow conc 5% Short Grass Pasture Kv= 7.0 fps
0.6	128	0.2500	3.50		Shallow Concentrated Flow, shallow conc 4H:1V Short Grass Pasture Kv= 7.0 fps
9.4	975	0.0010	1.73	62.22	Trap/Vee/Rect Channel Flow, channel flow Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.035 Earth, dense weeds
22.4	1,621	Total			

Subcatchment 1S: NW Drainage area Upstream culvert

Hydrograph



Summary for Subcatchment 6S: SE Drainage Area

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 46.08 cfs @ 12.16 hrs, Volume= 3.432 af, Depth> 2.18"

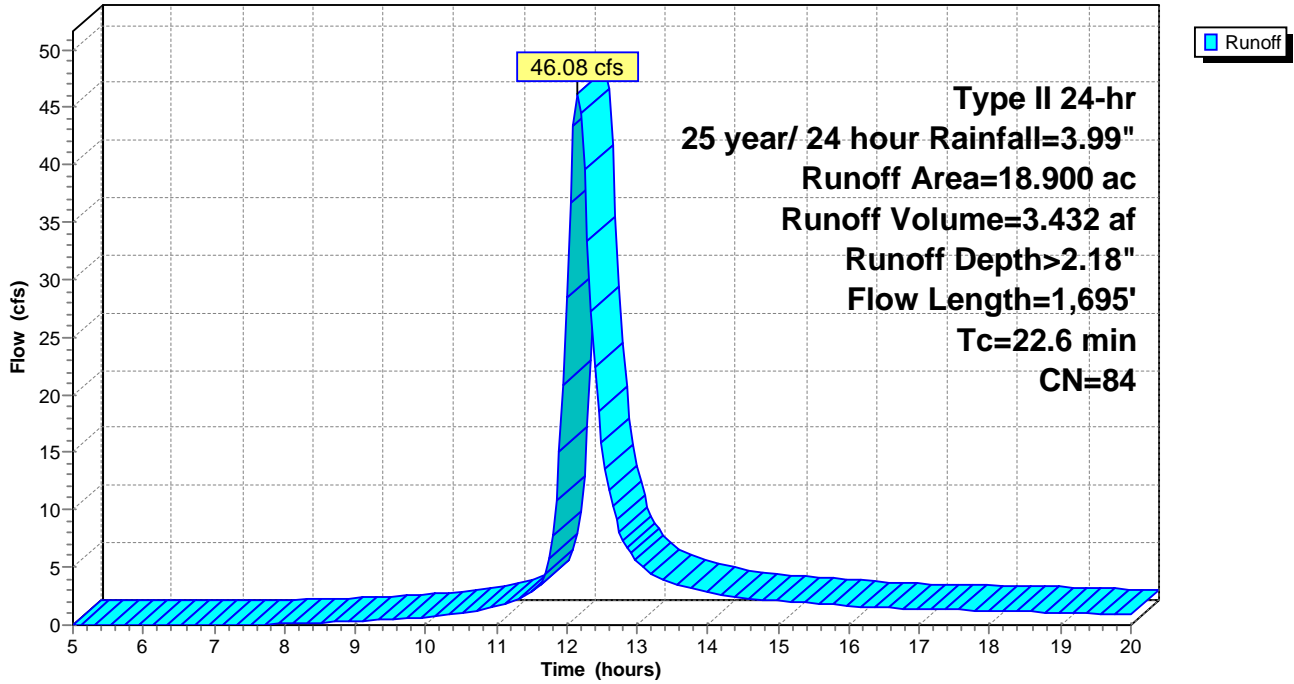
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 18.900	84	Clay cap with 6 inches of vegetative cover
18.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	100	0.0500	0.21		Sheet Flow, sheet flow 5% Grass: Short n= 0.150 P2= 2.35"
1.2	115	0.0500	1.57		Shallow Concentrated Flow, shallow conc 5% Short Grass Pasture Kv= 7.0 fps
0.7	155	0.2500	3.50		Shallow Concentrated Flow, shallow conc 4H:1V Short Grass Pasture Kv= 7.0 fps
12.8	1,325	0.0010	1.73	62.22	Trap/Vee/Rect Channel Flow, Channel flow Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.035 Earth, dense weeds
22.6	1,695	Total			

Subcatchment 6S: SE Drainage Area

Hydrograph



Summary for Subcatchment 9S: SW Drainage area

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 59.92 cfs @ 12.21 hrs, Volume= 5.004 af, Depth> 2.18"

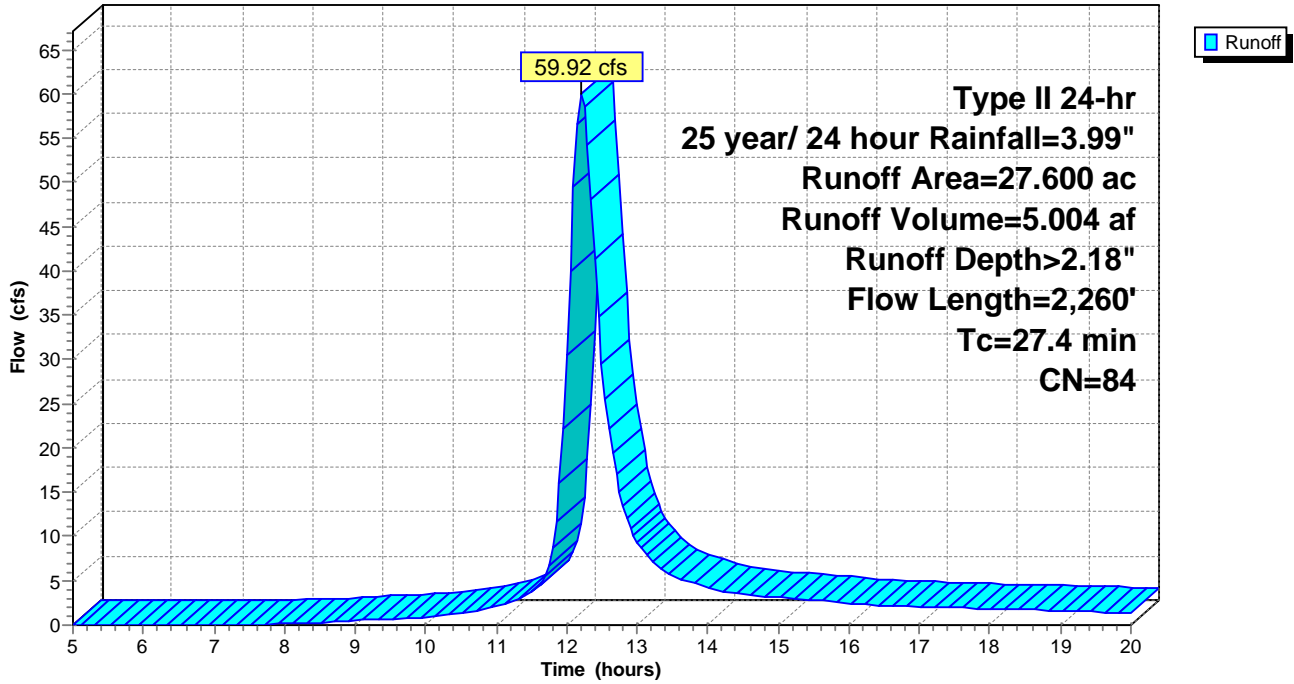
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 27.600	84	Clay cap with 6 inches of vegetative coer
27.600		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	100	0.0500	0.21		Sheet Flow, sheet flow 5% Grass: Short n= 0.150 P2= 2.35"
2.2	210	0.0500	1.57		Shallow Concentrated Flow, shallow conc 5% Short Grass Pasture Kv= 7.0 fps
0.7	140	0.2500	3.50		Shallow Concentrated Flow, shallow conc 4H:1V Short Grass Pasture Kv= 7.0 fps
16.6	1,810	0.0010	1.82	74.50	Trap/Vee/Rect Channel Flow, channel flow Bot.W=12.00' D=2.20' Z= 3.0 '/' Top.W=25.20' n= 0.035 Earth, dense weeds
27.4	2,260	Total			

Subcatchment 9S: SW Drainage area

Hydrograph



Summary for Subcatchment 12S: NE Drainage Area

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 41.32 cfs @ 12.33 hrs, Volume= 4.227 af, Depth> 2.17"

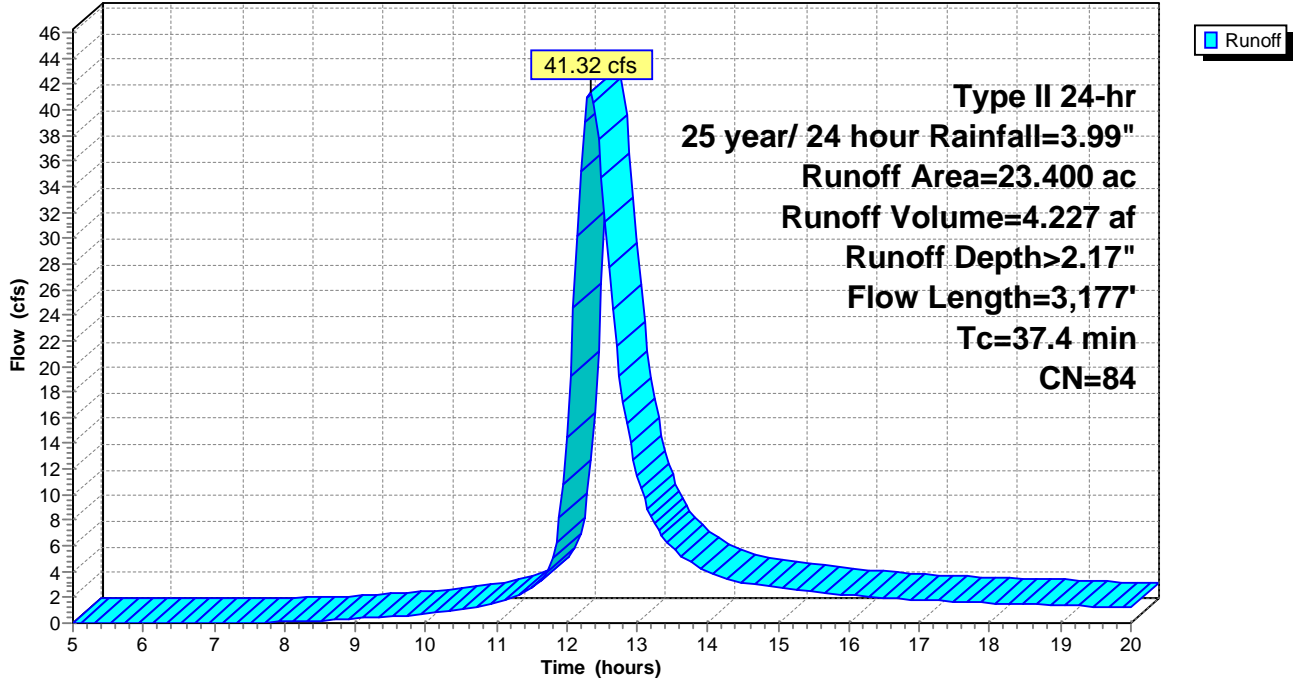
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 23.400	84	Clay cap with 6 inches of vegetative cover
23.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	100	0.0500	0.21		Sheet Flow, sheet flow 5% Grass: Short n= 0.150 P2= 2.35"
5.2	492	0.0500	1.57		Shallow Concentrated Flow, shallow conc 5% Short Grass Pasture Kv= 7.0 fps
0.6	130	0.2500	3.50		Shallow Concentrated Flow, shallow conc 4H:1V Short Grass Pasture Kv= 7.0 fps
23.7	2,455	0.0010	1.73	62.22	Trap/Vee/Rect Channel Flow, Channel flow Bot.W=12.00' D=2.00' Z= 3.0 '/' Top.W=24.00' n= 0.035 Earth, dense weeds
37.4	3,177	Total			

Subcatchment 12S: NE Drainage Area

Hydrograph



Summary for Subcatchment 15S: Ash Impoundment Drainage Area

Drainage area approximated based on survey completed in 2021. Design drawing did not include complete ash impoundment limits.

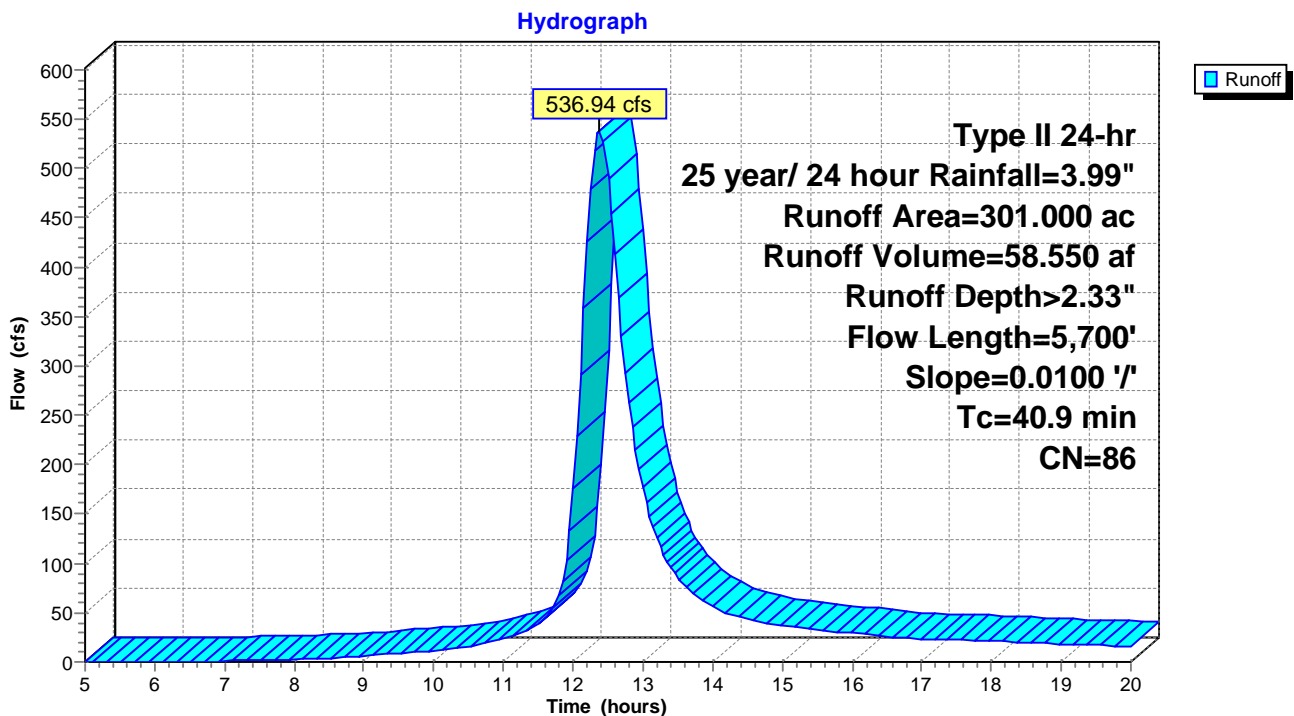
Runoff = 536.94 cfs @ 12.37 hrs, Volume= 58.550 af, Depth> 2.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 105.000	98	Lake
196.000	79	<50% Grass cover, Poor, HSG B
301.000	86	Weighted Average
196.000		65.12% Pervious Area
105.000		34.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	100	0.0100	0.11		Sheet Flow, sheet Grass: Short n= 0.150 P2= 2.35"
21.4	900	0.0100	0.70		Shallow Concentrated Flow, shallow concentrated f;pw Short Grass Pasture Kv= 7.0 fps
4.4	4,700		17.94		Lake or Reservoir, Lake Mean Depth= 10.00'
40.9	5,700	Total			

Subcatchment 15S: Ash Impoundment Drainage Area



Summary for Subcatchment 16S: NW Drainage area Downstream culvert

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 41.43 cfs @ 12.03 hrs, Volume= 2.191 af, Depth> 2.19"

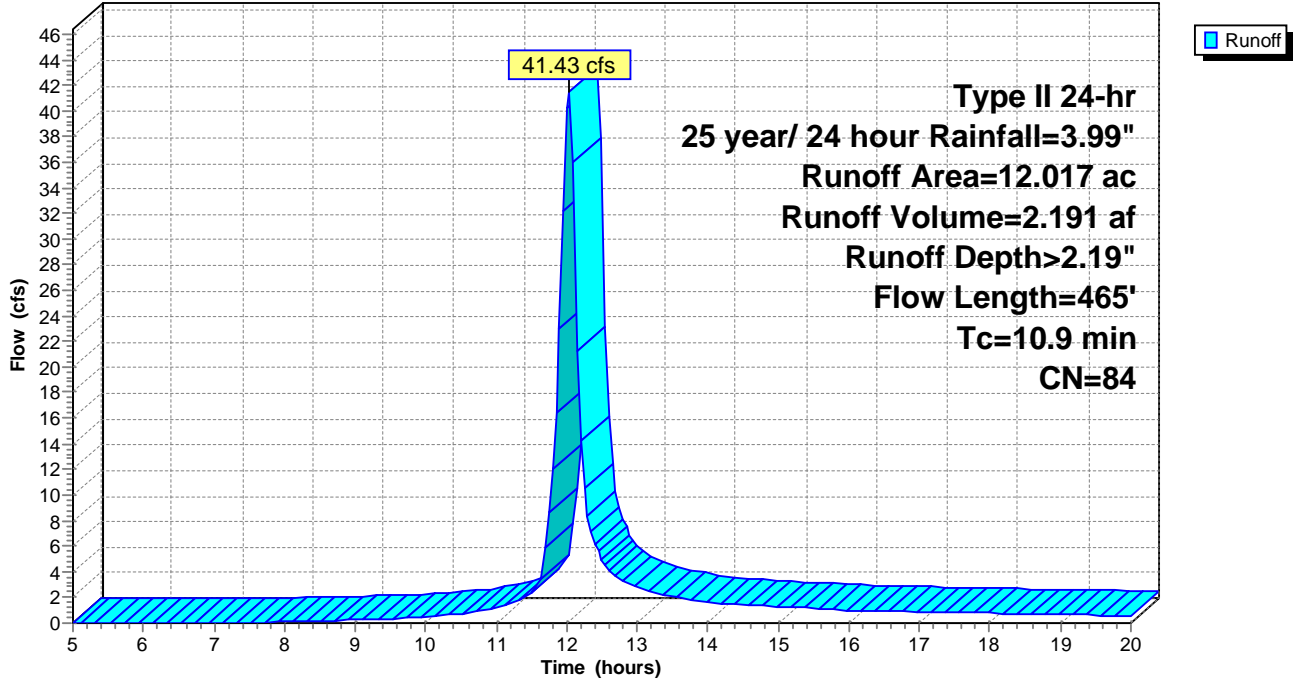
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 12.017	84	Clay cap with 6 inches of vegetative cover
12.017		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	100	0.0500	0.21		Sheet Flow, sheet flow 5% Grass: Short n= 0.150 P2= 2.35"
2.3	215	0.0500	1.57		Shallow Concentrated Flow, shallow conc 5% Short Grass Pasture Kv= 7.0 fps
0.7	150	0.2500	3.50		Shallow Concentrated Flow, shallow conc 4H:1V Short Grass Pasture Kv= 7.0 fps
10.9	465	Total			

Subcatchment 16S: NW Drainage area Downstream culvert

Hydrograph



Summary for Subcatchment 17S: Channel DA

Subcat area, flow length, and slope taken from Construction Permit Application Modification drawing set dated 3-20-2015 as well as a survey completed in 2021.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Length of each channel is included in the time of concentration for their respective subcats.

Exact design locations of the v-shaped channels created by the stormwater control berms are not available. In addition, in a storm event as large as the 25-year/24-hour event, it is assumed that they will be quickly exceeded, therefore they have not been included in this model.

Runoff = 4.11 cfs @ 12.00 hrs, Volume= 0.201 af, Depth> 2.19"

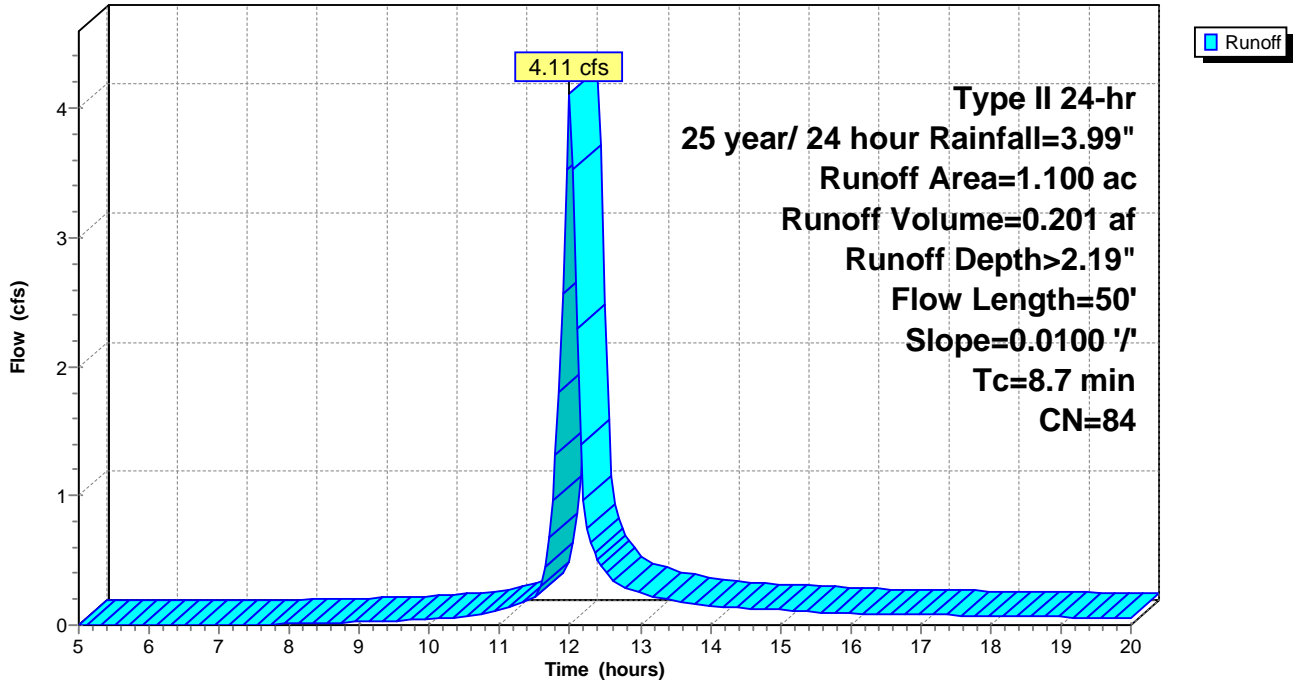
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 year/ 24 hour Rainfall=3.99"

Area (ac)	CN	Description
* 1.100	84	
1.100		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.7	50	0.0100	0.10		Sheet Flow, shallow conc Grass: Short n= 0.150 P2= 2.35"

Subcatchment 17S: Channel DA

Hydrograph



Summary for Reach 8R: NW Channel Section

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet invert taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 32.822 ac, 0.00% Impervious, Inflow Depth > 1.77" for 25 year/ 24 hour event
 Inflow = 34.47 cfs @ 12.20 hrs, Volume= 4.849 af
 Outflow = 35.36 cfs @ 12.10 hrs, Volume= 4.848 af, Atten= 0%, Lag= 0.0 min

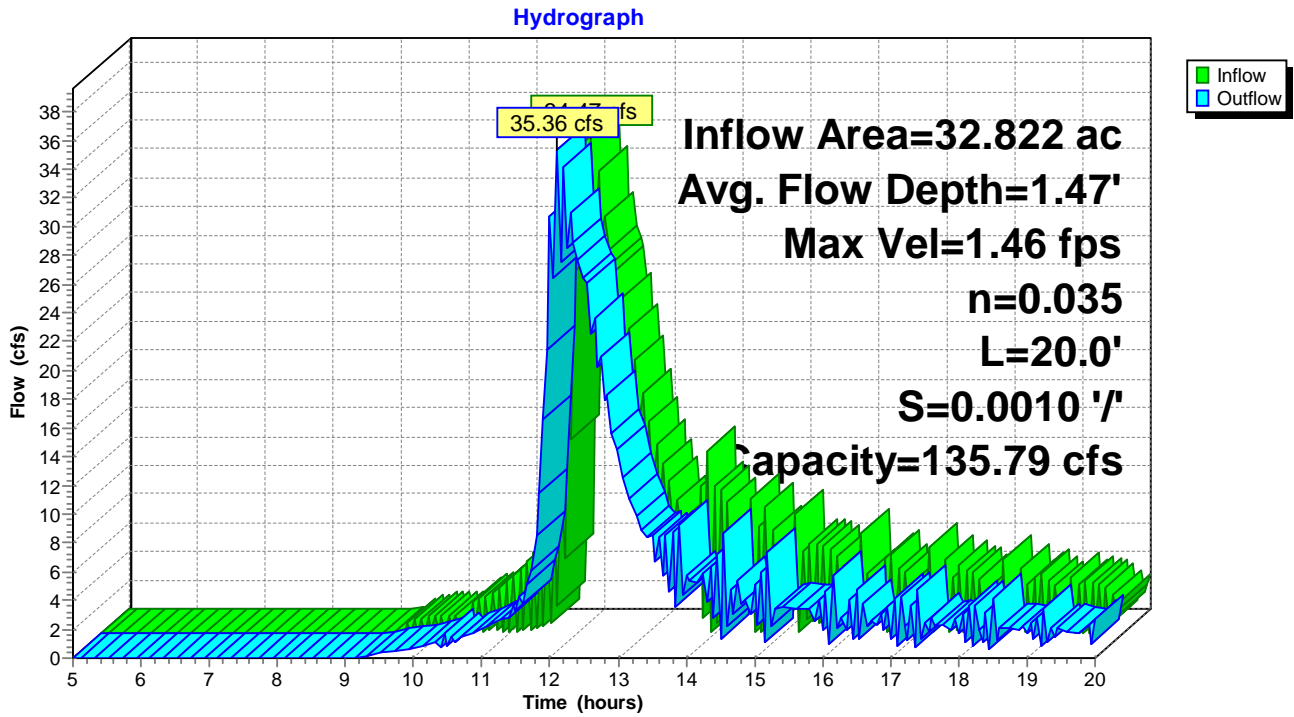
Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.46 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 0.58 fps, Avg. Travel Time= 0.6 min

Peak Storage= 484 cf @ 12.10 hrs
 Average Depth at Peak Storage= 1.47'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 135.79 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 '1' Top Width= 30.00'
 Length= 20.0' Slope= 0.0010 '1'
 Inlet Invert= 610.02', Outlet Invert= 610.00'



Reach 8R: NW Channel Section



Summary for Reach 9R: SW Channel Section

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet inverts taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 27.600 ac, 0.00% Impervious, Inflow Depth > 2.18" for 25 year/ 24 hour event
 Inflow = 59.92 cfs @ 12.21 hrs, Volume= 5.004 af
 Outflow = 59.92 cfs @ 12.21 hrs, Volume= 5.003 af, Atten= 0%, Lag= 0.1 min

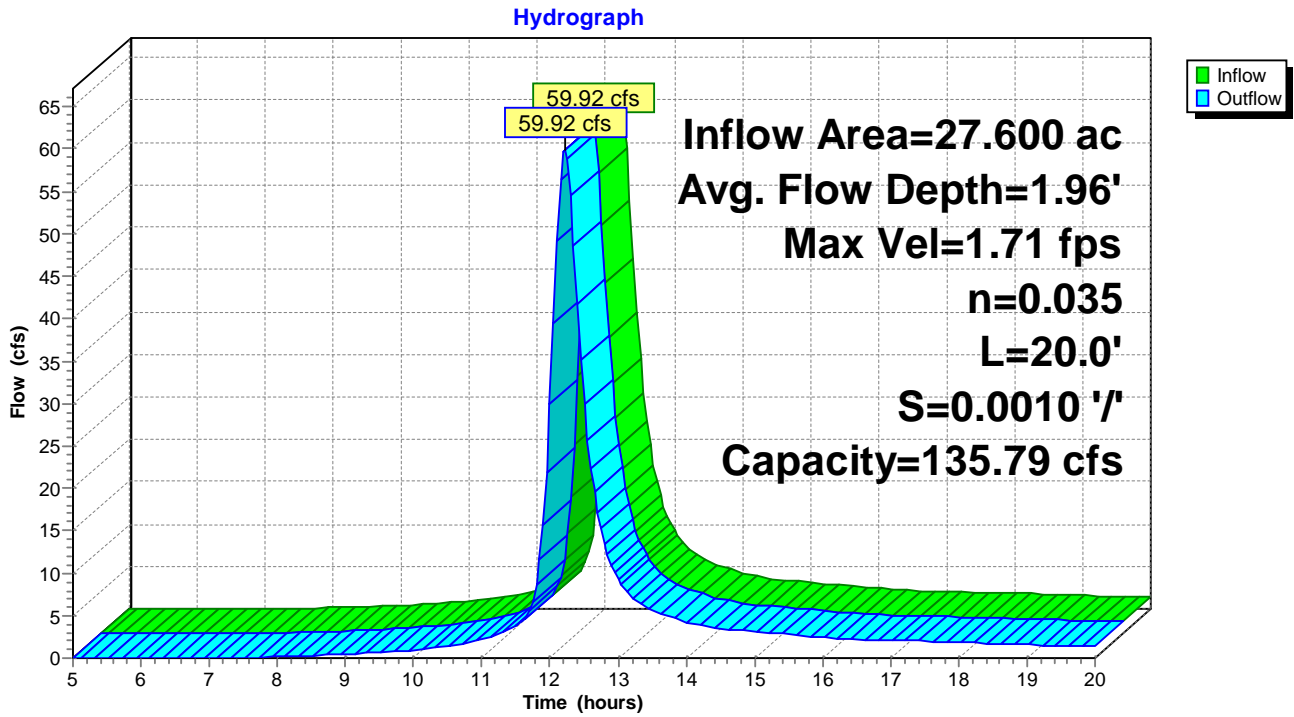
Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.71 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 0.58 fps, Avg. Travel Time= 0.6 min

Peak Storage= 701 cf @ 12.21 hrs
 Average Depth at Peak Storage= 1.96'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 135.79 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 ' / ' Top Width= 30.00'
 Length= 20.0' Slope= 0.0010 ' / '
 Inlet Invert= 610.02', Outlet Invert= 610.00'



Reach 9R: SW Channel Section



Summary for Reach 10R: Small channel

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet inverts taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

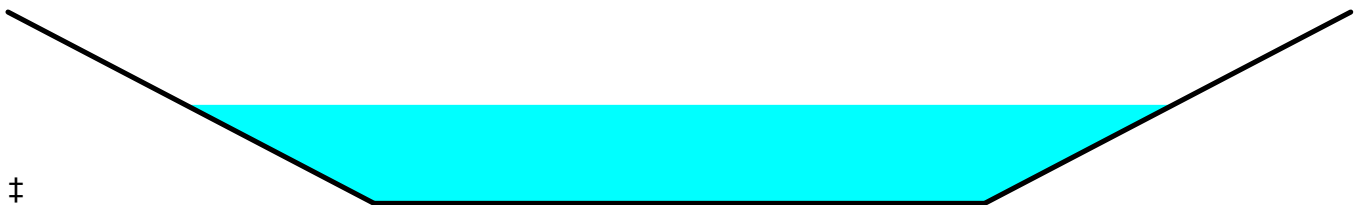
Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 46.500 ac, 0.00% Impervious, Inflow Depth > 1.63" for 25 year/ 24 hour event
 Inflow = 47.59 cfs @ 12.11 hrs, Volume= 6.297 af
 Outflow = 46.90 cfs @ 12.16 hrs, Volume= 6.275 af, Atten= 1%, Lag= 2.7 min

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.54 fps, Min. Travel Time= 2.4 min
 Avg. Velocity = 0.69 fps, Avg. Travel Time= 5.3 min

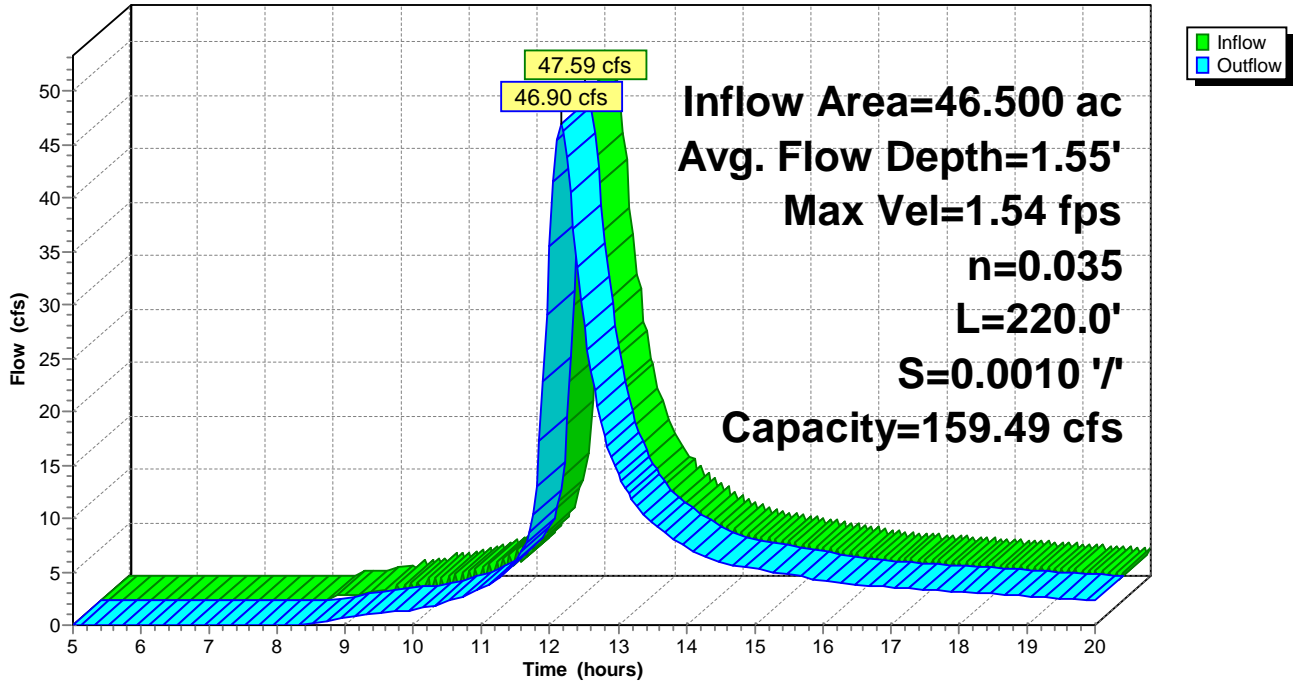
Peak Storage= 6,701 cf @ 12.16 hrs
 Average Depth at Peak Storage= 1.55'
 Bank-Full Depth= 3.00' Flow Area= 72.0 sf, Capacity= 159.49 cfs

15.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 '/' Top Width= 33.00'
 Length= 220.0' Slope= 0.0010 '/'
 Inlet Invert= 610.00', Outlet Invert= 609.78'



Reach 10R: Small channel

Hydrograph



Summary for Reach 12R: SE Channel Section

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet inverts taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 18.900 ac, 0.00% Impervious, Inflow Depth > 2.18" for 25 year/ 24 hour event
 Inflow = 46.08 cfs @ 12.16 hrs, Volume= 3.432 af
 Outflow = 46.08 cfs @ 12.16 hrs, Volume= 3.431 af, Atten= 0%, Lag= 0.1 min

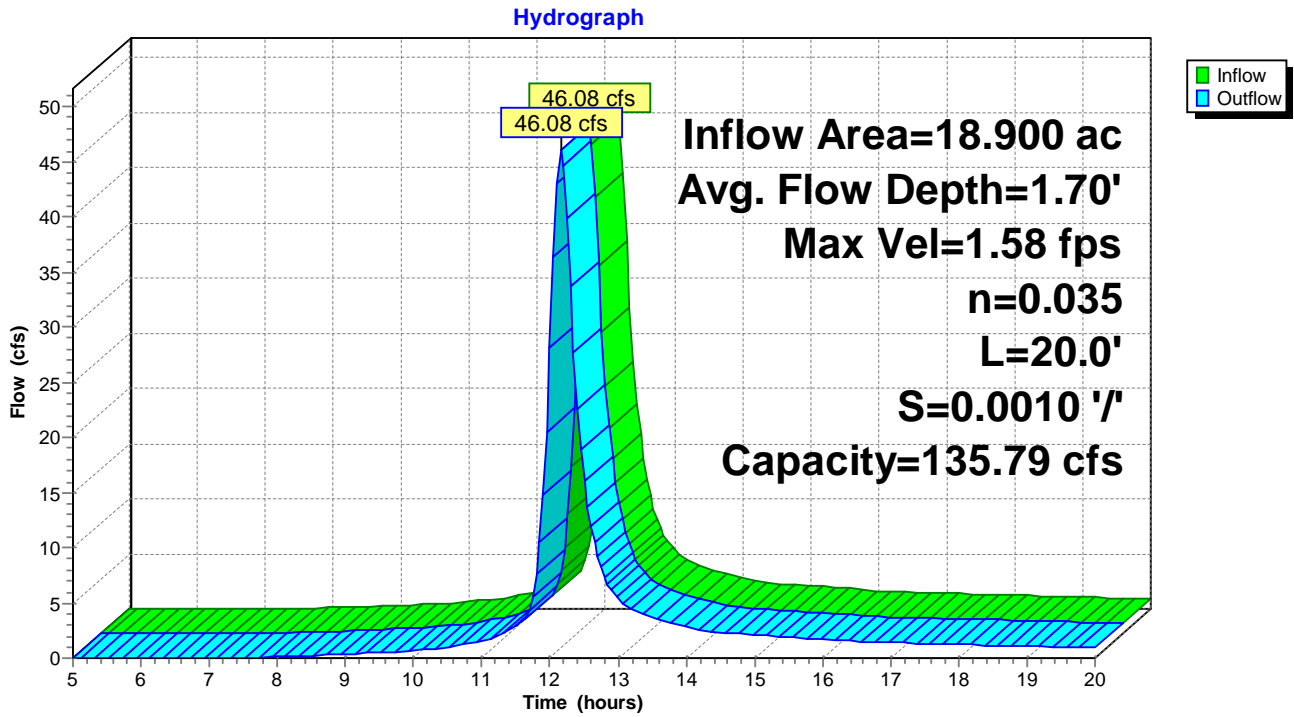
Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.58 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 0.51 fps, Avg. Travel Time= 0.7 min

Peak Storage= 583 cf @ 12.16 hrs
 Average Depth at Peak Storage= 1.70'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 135.79 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 ' / ' Top Width= 30.00'
 Length= 20.0' Slope= 0.0010 ' / '
 Inlet Invert= 610.02', Outlet Invert= 610.00'



Reach 12R: SE Channel Section



Summary for Reach 13R: NE Channel Section

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet inverts taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 23.400 ac, 0.00% Impervious, Inflow Depth > 2.17" for 25 year/ 24 hour event
 Inflow = 41.32 cfs @ 12.33 hrs, Volume= 4.227 af
 Outflow = 41.31 cfs @ 12.34 hrs, Volume= 4.226 af, Atten= 0%, Lag= 0.1 min

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.53 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 0.55 fps, Avg. Travel Time= 0.6 min

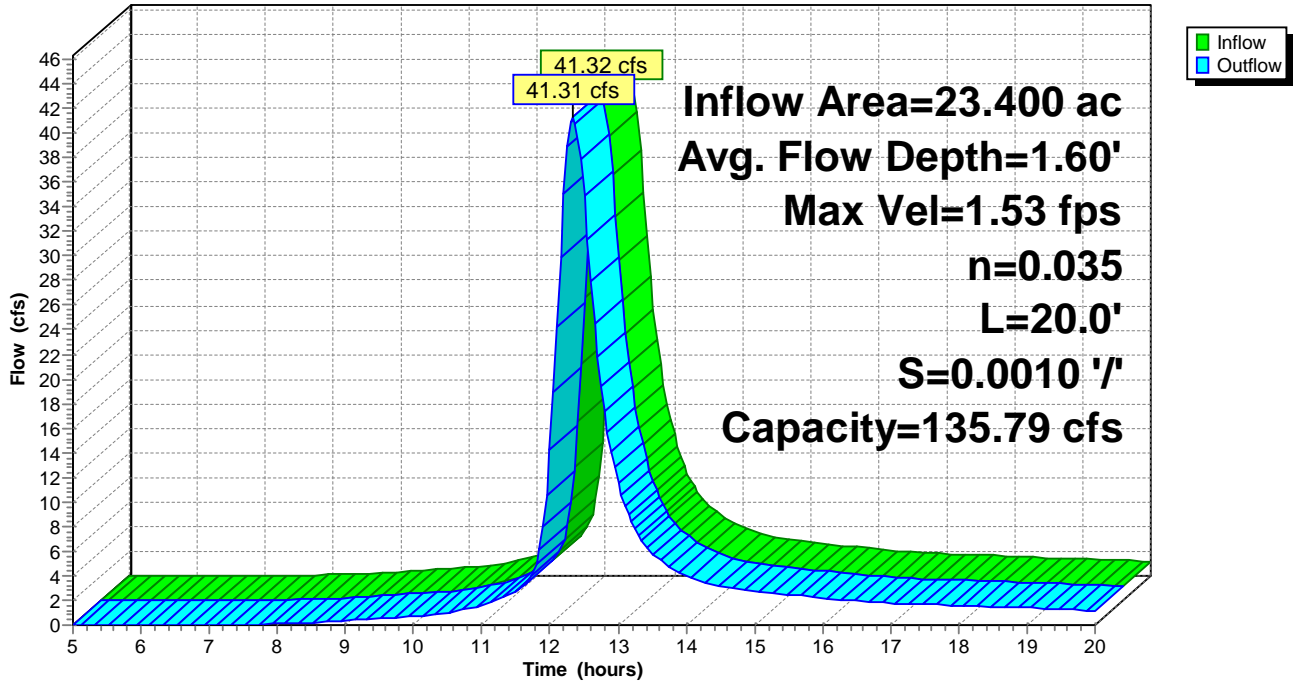
Peak Storage= 540 cf @ 12.34 hrs
 Average Depth at Peak Storage= 1.60'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 135.79 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 '/' Top Width= 30.00'
 Length= 20.0' Slope= 0.0010 '/'
 Inlet Invert= 609.34', Outlet Invert= 609.32'



Reach 13R: NE Channel Section

Hydrograph



Summary for Reach 14R: Swale between culverts

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet invert taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Inflow Area = 31.722 ac, 0.00% Impervious, Inflow Depth > 2.12" for 25 year/ 24 hour event
 Inflow = 76.94 cfs @ 12.06 hrs, Volume= 5.604 af
 Outflow = 53.79 cfs @ 12.19 hrs, Volume= 5.502 af, Atten= 30%, Lag= 7.8 min

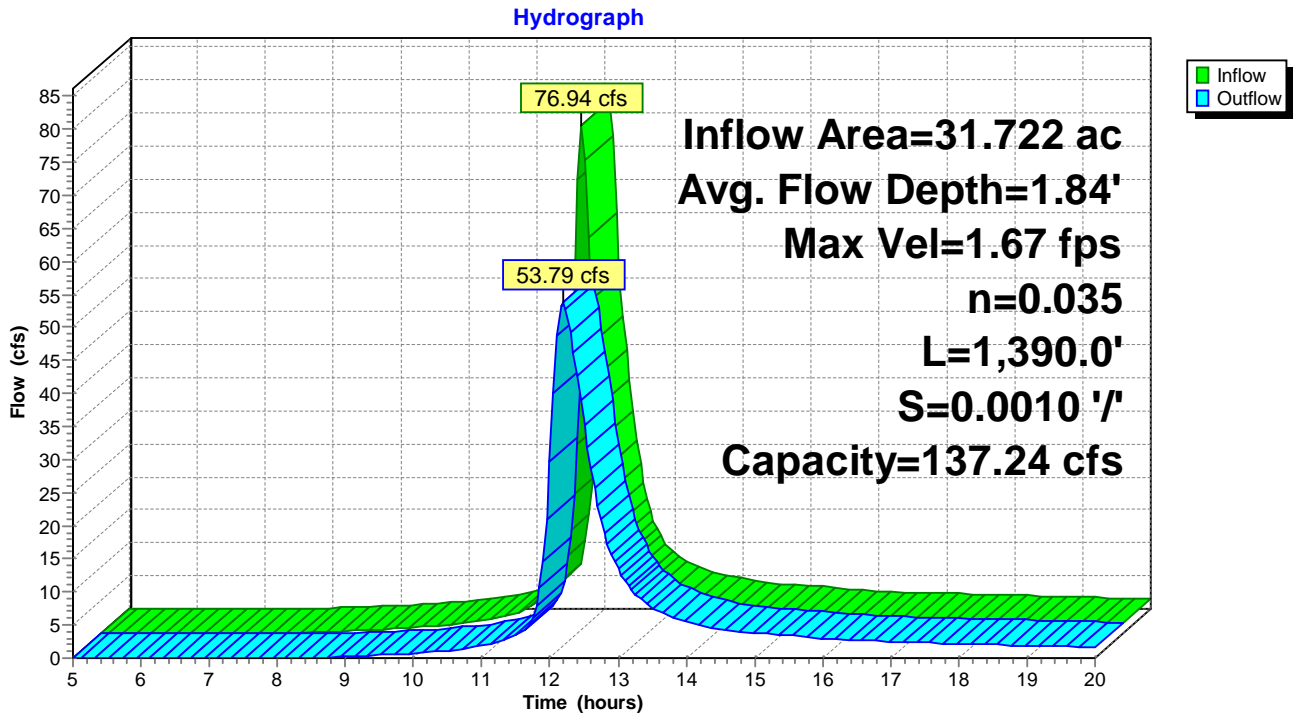
Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.67 fps, Min. Travel Time= 13.9 min
 Avg. Velocity = 0.60 fps, Avg. Travel Time= 38.4 min

Peak Storage= 44,803 cf @ 12.19 hrs
 Average Depth at Peak Storage= 1.84'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 137.24 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 '/' Top Width= 30.00'
 Length= 1,390.0' Slope= 0.0010 '/'
 Inlet Invert= 610.60', Outlet Invert= 609.18'



Reach 14R: Swale between culverts



Summary for Reach 15R: N Channel Section

Reach slopes of 0.1% and taken from Construction Permit Application Modification drawing set dated 3-20-2015 and outlet invert taken from 2021 survey.

General erosion matting was proposed which can vary. A manning's n value of 0.045 was used to simulate a grassed erosion matting. A lack of maintenance, or more robust erosion control measures may cause this value to increase potentially overflowing the channel..

Modeling only last 20 feet of channel as reach to check cross section for average resultant flow depths. The upstream portion of the reach is included in the subcat time of concentration.

Inflow Area = 19.705 ac, 0.00% Impervious, Inflow Depth > 2.18" for 25 year/ 24 hour event
 Inflow = 48.31 cfs @ 12.15 hrs, Volume= 3.579 af
 Outflow = 48.31 cfs @ 12.16 hrs, Volume= 3.578 af, Atten= 0%, Lag= 0.1 min

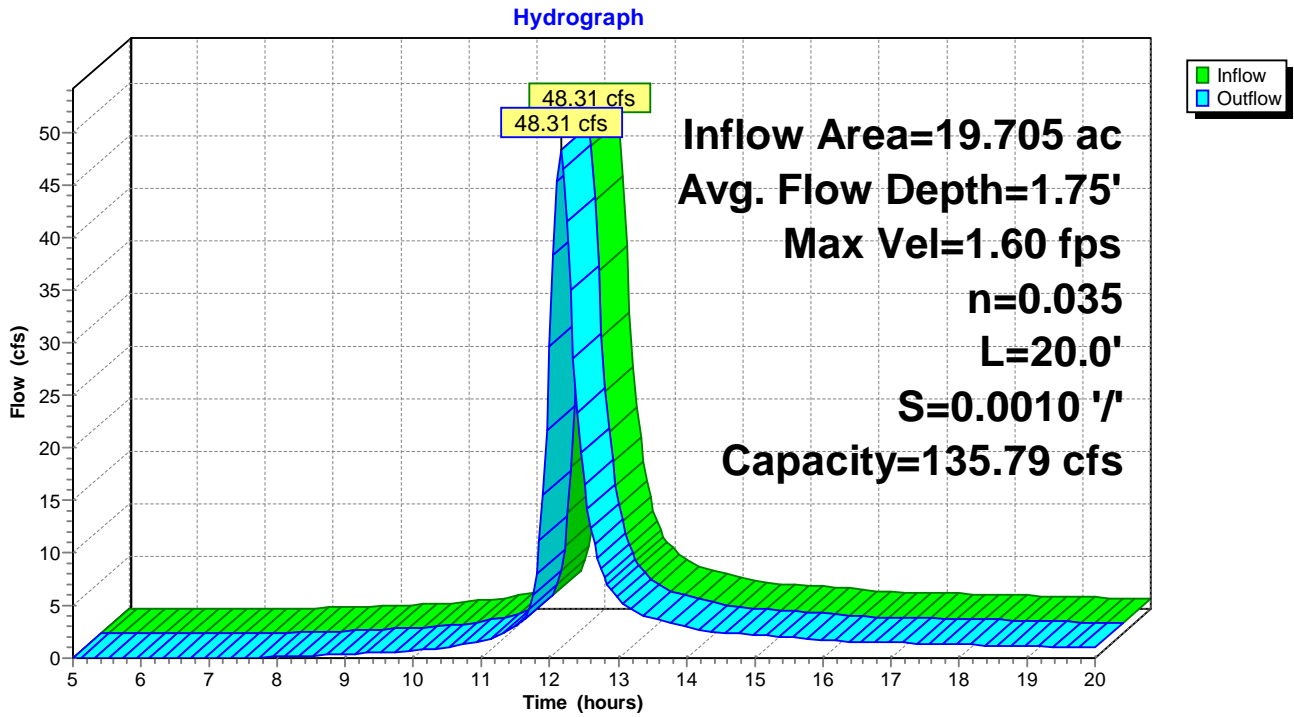
Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.60 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 0.52 fps, Avg. Travel Time= 0.6 min

Peak Storage= 602 cf @ 12.16 hrs
 Average Depth at Peak Storage= 1.75'
 Bank-Full Depth= 3.00' Flow Area= 63.0 sf, Capacity= 135.79 cfs

12.00' x 3.00' deep channel, n= 0.035 Earth, dense weeds
 Side Slope Z-value= 3.0 '/' Top Width= 30.00'
 Length= 20.0' Slope= 0.0010 '/'
 Inlet Invert= 610.62', Outlet Invert= 610.60'



Reach 15R: N Channel Section



Summary for Pond 7P: Ash Impoundment

Storage was taken from bathymetric contours in AutoCAD file from Construction Permit Application Modification drawing set dated 3-20-2015 provided by DTE.

Starting WSE and outfall information was determined based on design drawings titled "Discharge structure - existing & proposed conditions" by Detroit Edison's surveying services dated 6-20-2016 and "Layout - on-site Fly ash Disposal facility discharge structure wastewater treatment plant" by Commonwealth Associates Inc. dated 6-23-1977.

Inflow Area = 403.722 ac, 26.01% Impervious, Inflow Depth > 3.06" for 25 year/ 24 hour event
 Inflow = 665.60 cfs @ 12.36 hrs, Volume= 102.867 af, Incl. 23.29 cfs Base Flow
 Outflow = 65.51 cfs @ 15.65 hrs, Volume= 54.825 af, Atten= 90%, Lag= 197.4 min
 Primary = 65.51 cfs @ 15.65 hrs, Volume= 54.825 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Starting Elev= 609.00' Surf.Area= 154.000 ac Storage= 152.500 af
 Peak Elev= 609.34' @ 15.65 hrs Surf.Area= 154.677 ac Storage= 204.707 af (52.207 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 65.2 min (852.9 - 787.7)

Volume	Invert	Avail.Storage	Storage Description
#1	608.00'	307.500 af	Custom Stage Data (Prismatic) Listed below (Recalc)

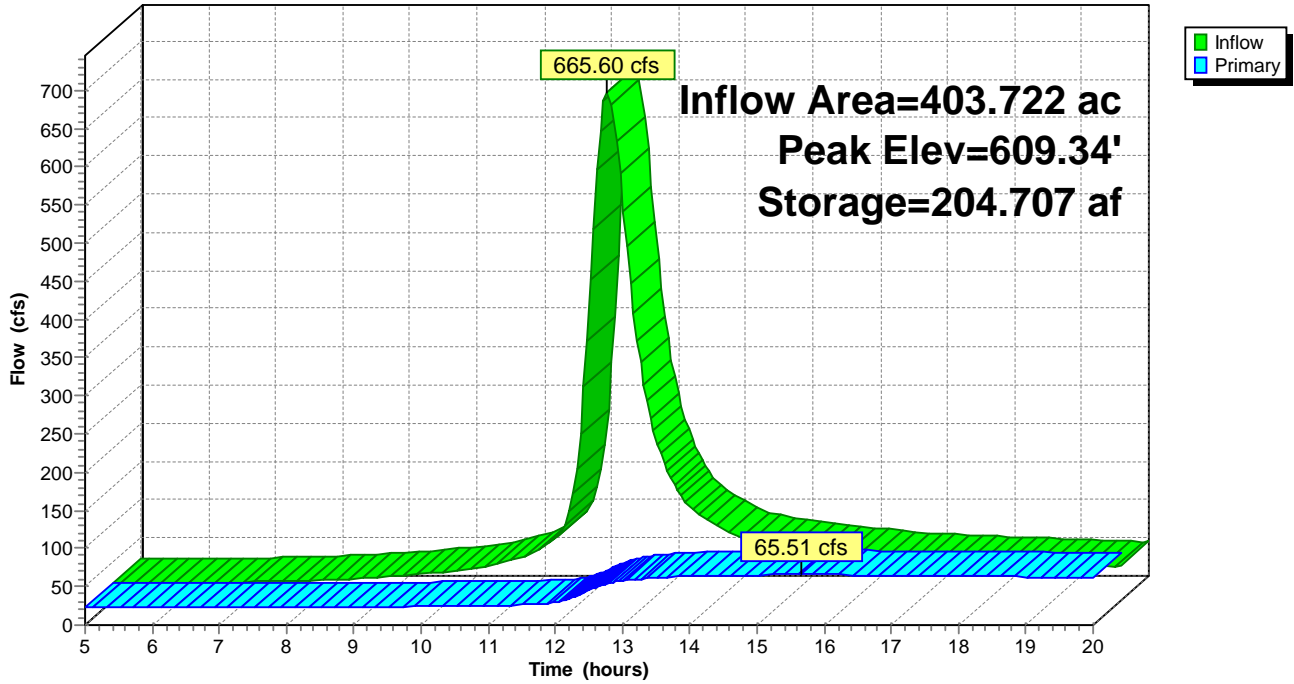
Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
608.00	151.000	0.000	0.000
609.00	154.000	152.500	152.500
610.00	156.000	155.000	307.500

Device	Routing	Invert	Outlet Devices
#1	Primary	605.50'	36.0" Round Culvert X 3.00 L= 150.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 605.50' / 576.75' S= 0.1917 '/' Cc= 0.900 n= 0.012 Steel, smooth, Flow Area= 7.07 sf
#2	Device 1	608.66'	36.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Primary	613.00'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 1.50 Width (feet) 0.00 200.00

Primary OutFlow Max=65.51 cfs @ 15.65 hrs HW=609.34' (Free Discharge)
 1=Culvert (Passes 65.51 cfs of 156.13 cfs potential flow)
 2=Sharp-Crested Rectangular Weir (Weir Controls 65.51 cfs @ 2.69 fps)
 3=Custom Weir/Orifice (Controls 0.00 cfs)

Pond 7P: Ash Impoundment

Hydrograph



Summary for Pond 8P: Reach confluence

Modeled as pond to see if the confluence of the two reaches overflows.

Inflow Area = 46.500 ac, 0.00% Impervious, Inflow Depth > 2.18" for 25 year/ 24 hour event
 Inflow = 104.55 cfs @ 12.19 hrs, Volume= 8.434 af
 Outflow = 104.96 cfs @ 12.19 hrs, Volume= 8.412 af, Atten= 0%, Lag= 0.3 min
 Primary = 47.59 cfs @ 12.11 hrs, Volume= 6.297 af
 Secondary = 59.09 cfs @ 12.21 hrs, Volume= 2.115 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 611.56' @ 12.21 hrs Surf.Area= 0.028 ac Storage= 0.052 af

Plug-Flow detention time= 2.1 min calculated for 8.412 af (100% of inflow)
 Center-of-Mass det. time= 1.1 min (794.1 - 793.0)

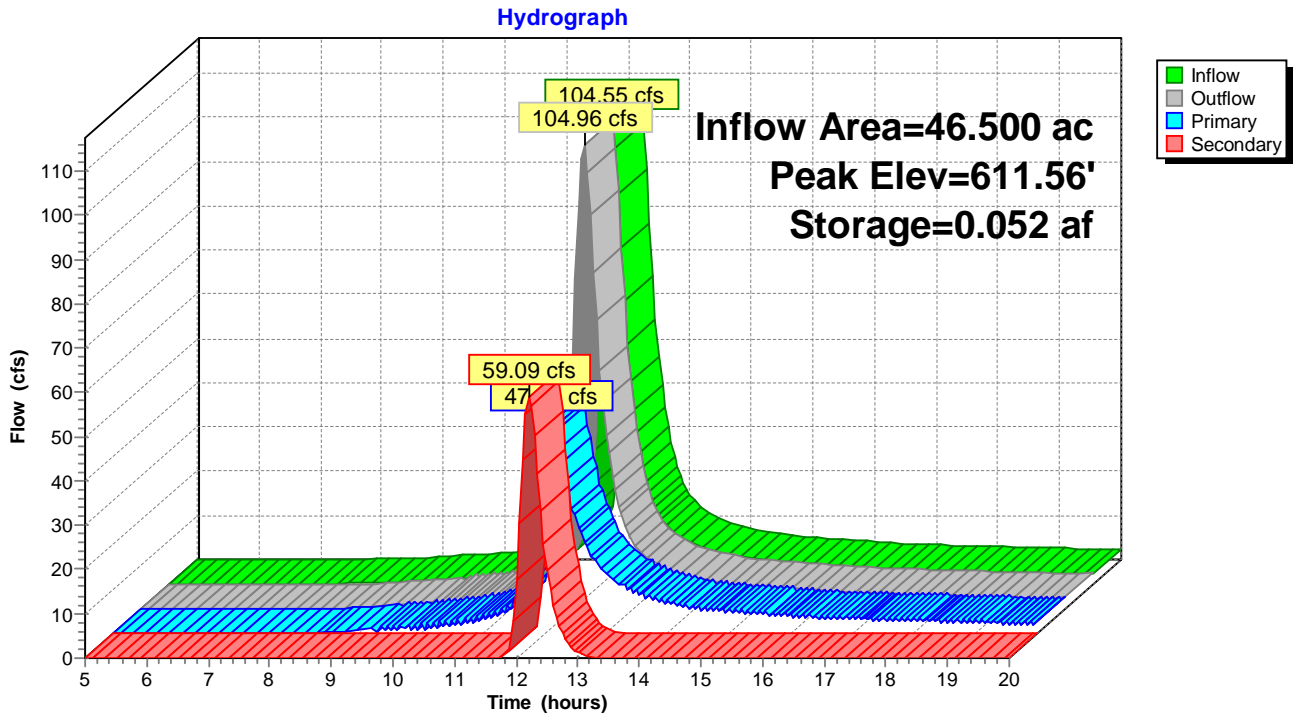
Volume	Invert	Avail.Storage	Storage Description
#1	607.60'	0.053 af	12.00'W x 10.00'L x 4.00'H Prismaoid Z=3.0

Device	Routing	Invert	Outlet Devices
#1	Primary	607.60'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 4.00 Width (feet) 12.00 36.00
#2	Secondary	610.60'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 1.00 Width (feet) 0.00 50.00

Primary OutFlow Max=0.00 cfs @ 12.11 hrs HW=611.48' TW=611.53' (Dynamic Tailwater)
 ↑1=Custom Weir/Orifice (Controls 0.00 cfs)

Secondary OutFlow Max=58.71 cfs @ 12.21 hrs HW=611.56' (Free Discharge)
 ↑2=Custom Weir/Orifice (Weir Controls 58.71 cfs @ 2.56 fps)

Pond 8P: Reach confluence



Summary for Pond 10P: Culvert test

Modeled as pond to see if the confluence of the two reaches overflows.

Inflow Area = 31.722 ac, 0.00% Impervious, Inflow Depth > 2.08" for 25 year/ 24 hour event
 Inflow = 53.79 cfs @ 12.19 hrs, Volume= 5.502 af
 Outflow = 55.38 cfs @ 12.19 hrs, Volume= 5.488 af, Atten= 0%, Lag= 0.5 min
 Primary = 33.52 cfs @ 12.20 hrs, Volume= 4.649 af
 Secondary = 25.87 cfs @ 12.16 hrs, Volume= 0.839 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 611.74' @ 12.16 hrs Surf.Area= 0.025 ac Storage= 0.045 af

Plug-Flow detention time= 2.3 min calculated for 5.488 af (100% of inflow)
 Center-of-Mass det. time= 1.4 min (807.2 - 805.9)

Volume	Invert	Avail.Storage	Storage Description
#1	608.05'	0.053 af	12.00'W x 10.00'L x 4.00'H Prismatic Z=3.0

Device	Routing	Invert	Outlet Devices
#1	Primary	608.05'	36.0" Round Culvert X 2.00 L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 608.05' / 608.01' S= 0.0008 '/ Cc= 0.900 n= 0.011, Flow Area= 7.07 sf
#2	Secondary	611.05'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 1.00 Width (feet) 0.00 50.00

Primary OutFlow Max=25.86 cfs @ 12.20 hrs HW=611.69' TW=611.46' (Dynamic Tailwater)

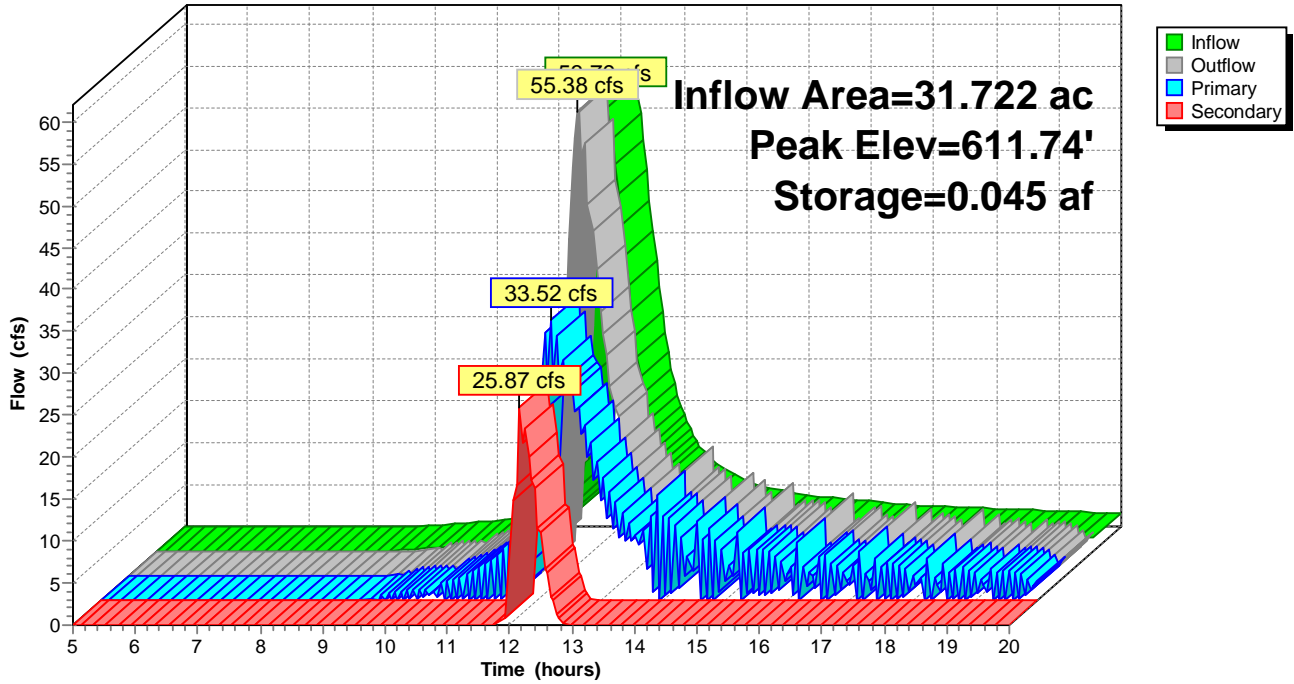
↑**1=Culvert** (Inlet Controls 25.86 cfs @ 1.83 fps)

Secondary OutFlow Max=24.82 cfs @ 12.16 hrs HW=611.73' (Free Discharge)

↑**2=Custom Weir/Orifice** (Weir Controls 24.82 cfs @ 2.16 fps)

Pond 10P: Culvert test

Hydrograph



Summary for Pond 11P: Culvert test

Modeled as pond to see if the confluence of the two reaches overflows.

Inflow Area = 19.705 ac, 0.00% Impervious, Inflow Depth > 2.18" for 25 year/ 24 hour event
 Inflow = 48.31 cfs @ 12.16 hrs, Volume= 3.578 af
 Outflow = 48.31 cfs @ 12.16 hrs, Volume= 3.570 af, Atten= 0%, Lag= 0.2 min
 Primary = 40.16 cfs @ 12.11 hrs, Volume= 3.413 af
 Secondary = 10.47 cfs @ 12.19 hrs, Volume= 0.157 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 612.88' @ 12.19 hrs Surf.Area= 0.023 ac Storage= 0.038 af

Plug-Flow detention time= 1.9 min calculated for 3.570 af (100% of inflow)
 Center-of-Mass det. time= 1.1 min (791.7 - 790.6)

Volume	Invert	Avail.Storage	Storage Description
#1	609.47'	0.053 af	12.00'W x 10.00'L x 4.00'H Prismaoid Z=3.0

Device	Routing	Invert	Outlet Devices
#1	Primary	609.40'	36.0" Round Culvert X 2.00 L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 609.40' / 609.36' S= 0.0008 '/ Cc= 0.900 n= 0.011, Flow Area= 7.07 sf
#2	Secondary	612.40'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 1.00 Width (feet) 0.00 50.00

Primary OutFlow Max=33.61 cfs @ 12.11 hrs HW=612.75' TW=612.36' (Dynamic Tailwater)

↑**1=Culvert** (Inlet Controls 33.61 cfs @ 2.38 fps)

Secondary OutFlow Max=10.20 cfs @ 12.19 hrs HW=612.88' (Free Discharge)

↑**2=Custom Weir/Orifice** (Weir Controls 10.20 cfs @ 1.81 fps)

Pond 11P: Culvert test

Hydrograph

